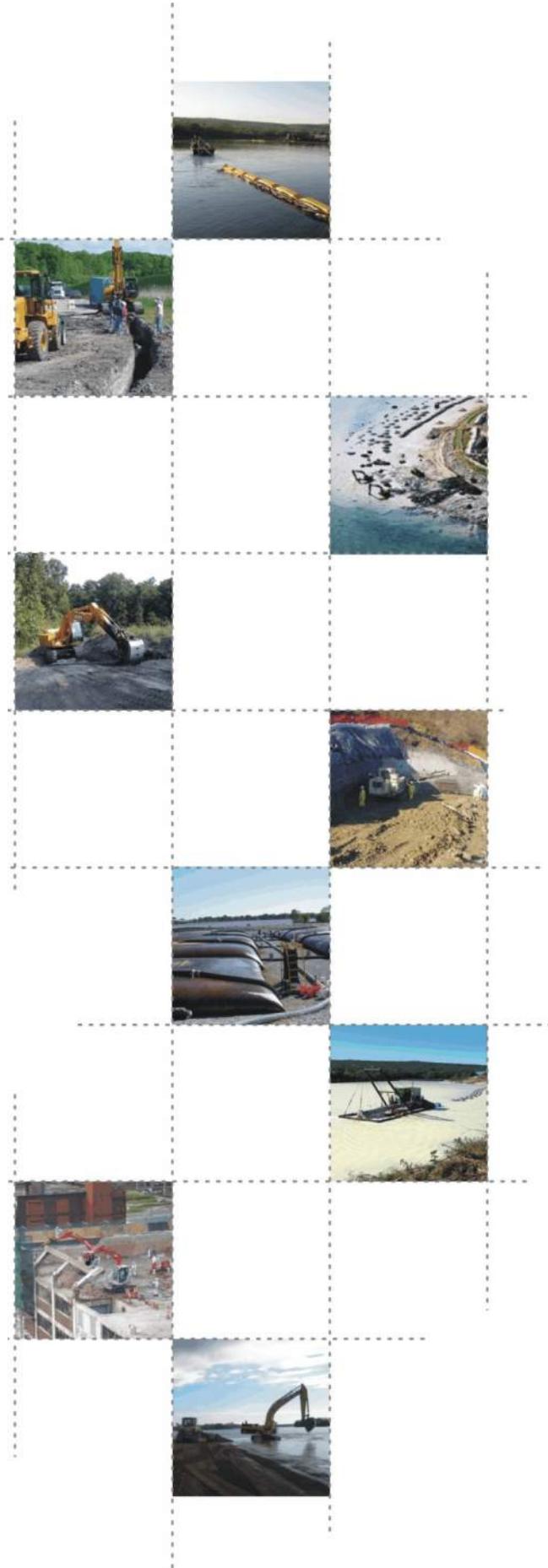


ALLIANT ENERGY
Interstate Power and Light Company
Burlington Generating Station

CCR SURFACE IMPOUNDMENT

INFLOW DESIGN FLOOD CONTROL PLAN

Report Issued: July 26, 2017
Revision 1



EXECUTIVE SUMMARY

This Inflow Flood Control Plan (Report) is prepared in accordance with the requirements of the United States Environmental Protection Agency (USEPA) published Final Rule for Hazardous and Solid Waste Management System - Disposal of Coal Combustion Residual from Electric Utilities (40 CFR Parts 257 and 261, also known as the CCR Rule) published on April 17, 2015 and effective October 19, 2015.

This Report assesses the hydrologic and hydraulic capacity requirements for each CCR unit at Burlington Generating Station in Burlington, Iowa in accordance with §257.82 of the CCR Rule. For purposes of this Report, a CCR unit is defined as any existing CCR surface impoundment. Primarily, the Report documents how the inflow design flood control system has been designed and constructed to meet the CCR Rule section §257.82.



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1 INTRODUCTION

The owner or operator of the Coal Combustion Residual (CCR) unit must conduct an initial and periodic inflow design flood control system plan to determine if each CCR unit adequately manages flow into and from each CCR unit during and following the peak discharge of the inflow design flood. The inflow design flood is selected based on the hazard potential classification (§257.73(a)(2)) for each CCR unit.

The Report has been prepared in accordance with the requirements of §257.82 of the CCR Rule.

1.1 CCR Rule Applicability

The CCR Rule requires an initial and periodic inflow design flood control system plan certified by a qualified professional engineer (PE) for all existing CCR surface impoundments. This report is a revision to the initial inflow design flood control system plan.

1.2 Hydrologic and Hydraulic Capacity Applicability

The Burlington Generating Station (BGS) in Burlington, Iowa (Figure 1) has four existing CCR surface impoundments that meet the requirements of §257.73(b)(1) or §257.73(b)(2) of the CCR Rule, which are identified as follows:

- BGS Ash Seal Pond
- BGS Main Ash Pond
- BGS Economizer Pond
- BGS Upper Ash Pond



2 FACILITY DESCRIPTION

The following sub-section provides a summary description of the facility and existing CCR surface impoundments located at BGS.

BGS is located southeast of the City of Burlington, Iowa on the western shore of the Mississippi River in Des Moines County, at 4282 Sullivan Slough Road, Burlington, Iowa (Figure 1). BGS is a fossil-fueled electric generating station consisting of one steam electric generating unit and four combustion turbine units. Sub-bituminous coal is the primary fuel for producing steam, with the ability to use natural gas for the combustion turbines. The burning of coal produces a by-product of CCR. The CCR at BGS is categorized into three types: bottom ash, economizer ash, and precipitator fly ash.

General Facility Information:

Date of Initial Facility Operations: 1968

NPDES Permit Number: IA29-00-1-01

Facility Title V Operating Permit: 98-TV-023R1-M004

Latitude / Longitude: 40°44'29"N 91°07'04"W

Site Coordinates: Section 29, Township 69 North, Range 02 West

Unit Nameplate Ratings: Unit 1: 212 MW

2.1 BGS Ash Seal Pond

The BGS Ash Seal Pond is located south of the generating plant and east of the BGS Main Ash Pond. The CCR, in 1968, was originally managed by discharging into the BGS Ash Seal Pond for settling. Presently, the BGS Ash Seal Pond only receives storm water runoff from the surrounding area associated with the fly ash storage silo. The BGS Ash Seal Pond also may receive facility process water, such as ash seal water, but only if there is an issue with the ash seal water pumps. At the time of the initial annual inspection on October 26, 2015 this CCR surface impoundment did not contain standing water.



The surface area of the BGS Ash Seal Pond is approximately 5.7 acres and has an embankment height of approximately 12 feet from the crest to the toe of the downstream slope. The embankment crest is at elevation 534 the same as the adjacent plant site grade and equivalent to the 100 year flood water elevation of the Mississippi River. The interior storage depth of the BGS Ash Seal Pond is approximately 12 feet. If water were present, the total volume of impounded CCR and water within the BGS Ash Seal Pond would be approximately 97,000 cubic yards, which would include general fill that has been added in the northeast corner of the impoundment. The original outfall for the impoundment is sealed to prevent discharge to the Mississippi River and the impoundment normally contains no water. Rainfall that accumulates exfiltrates through the bottom of the impoundment. A manually operated pump is available to lift storm water to the adjacent BGS Main Ash Pond, if necessary.

2.2 BGS Main Ash Pond

The BGS Main Ash Pond is located southwest of the generating plant and west of the BGS Ash Seal Pond. The CCR, prior to being sluiced to the BGS Main Ash Pond, was originally managed in the BGS Ash Seal Pond in 1968. In 1971, BGS managed CCR in the BGS Upper Ash Pond. In 1980, the BGS Main Ash Pond became the primary receiver of CCR, with the BGS Upper Ash Pond becoming a downstream receiver.

Presently, the BGS Main Ash Pond receives bottom ash that is sluiced from the generating plant to the northeast corner of the BGS Main Ash Pond. The sluiced bottom ash discharges into the northeast corner where the majority of the bottom ash settles out. The bottom ash that settles out is recovered for beneficial reuse. Hydrated fly ash is also stored within the BGS Main Ash Pond area prior to being sold as aggregate material for beneficial reuse. Fly ash from the on-site storage silo is no longer added to the embankment.

The water that is used to sluice the bottom ash into the BGS Main Ash Pond is routed towards the west end of the BGS Main Ash Pond. The water is discharged in batch



quantities as bottom ash accumulates in the boiler and averages 1 cubic foot per second (cfs) on a daily basis. The water flows to the west along the north side of a road constructed out of bottom ash through the center of the BGS Main Ash Pond, Figure 2. The water flows along the north side of the road until it reaches the west end where it transitions into a ponded area in the northwest corner of the BGS Main Ash Pond. The water in the northwest corner of the BGS Main Ash Pond flows through two 15-inch diameter corrugated metal culverts with identical invert elevation under the generating plant entrance road. The water discharges into a small channel in the southwest corner of the BGS Upper Ash Pond located north of the generating plant entrance road.

The surface area of the BGS Main Ash Pond is approximately 18.7 acres and has an embankment height of approximately 12 feet from the crest to the toe of the downstream slope. The embankment crest is at elevation 534 the same as the plant site grade and equivalent to the 100 year flood water elevation in the Mississippi River. The interior storage depth of the BGS Main Ash Pond is approximately 8 feet. The total volume of impounded CCR and water within the BGS Main Ash Pond at normal water operation elevation is approximately 240,000 cubic yards. Additional volume of impounded CCR, located in the eastern half of the BGS Main Ash Pond above the crest elevation of the embankment, includes the bottom ash storage area and C-stone embankment (hydrated fly ash). In 2008, the quantity of the additional CCR above the crest elevation of the embankment is approximately 104,000 cubic yards.

2.3 BGS Economizer Pond

The BGS Economizer Pond is located west of the generating plant and north of the BGS Main Ash Pond. In 1986, BGS constructed the BGS Economizer Pond in the southern and eastern portion of the original footprint of the BGS Upper Ash Pond. The impoundment has resulted from economizer ash that has been deposited since 1986, which created the economizer embankment which is higher than the embankments of the BGS Upper Ash Pond at approximately elevation 548.



Presently, the BGS Economizer Pond receives economizer ash. The economizer ash is sluiced from the generating plant to the east end of the BGS Economizer Pond via a 10-inch diameter polyvinyl chloride pipe at a flow rate of 1.5 cfs (including approximately 10% plant process water). The economizer ash settles out through the water column of the 0.4 acre BGS Economizer Pond while the water flows to the west. The water discharges from the BGS Economizer Pond through an 18-inch diameter high-density polyethylene pipe into a storm water and process water treatment channel located along the south side of the economizer embankment.

The storm water and process water treatment channel receives runoff from 8 acres surrounding the generating plant. The collected storm water drains into a pump vault located at the toe of the downstream slope of the east embankment of the BGS Economizer Pond. Plant process water flows through an oil/water separator and receives influent flows from the plant floor drains and water treatment process water. After the oil/water separator, the process water discharges into the pump vault. The storm water and process water is then pumped from the vault up to the storm water treatment channel. The storm water treatment channel flows to the west along the south side of the economizer embankment until it discharges through an 18-inch diameter high-density polyethylene pipe located in the southwest corner of the economizer embankment. The water from the storm water treatment channel discharges into a small channel in the southwest corner of the BGS Upper Ash Pond located north of the generating plant entrance road.

The total surface area of the BGS Economizer Pond and economizer embankment is approximately 11 acres and has an embankment height of approximately 13 feet from the crest to the toe of slope on the CCR in the BGS Upper Ash Pond. The interior storage depth of the top of the economizer embankment to the bottom of the original footprint of the BGS Upper Ash Pond is approximately 27 feet. Thus, the total volume of impounded CCR and water within the BGS Economizer Pond including CCR already in place when the impoundment was established is approximately 480,000 cubic yards.



2.4 BGS Upper Ash Pond

The BGS Upper Ash Pond is located northwest of the generating plant and north of the BGS Main Ash Pond. In 1971, BGS began managing CCR in the BGS Upper Ash Pond. In 1980, the BGS Main Ash Pond became the primary receiver of CCR and the BGS Upper Ash Pond became a downstream receiver of the BGS Main Ash Pond.

Presently, the BGS Upper Ash Pond receives influent flows from the BGS Main Ash Pond, BGS Economizer Pond, and storm water and process water flow from the generating plant. The influent flows all discharge into a small channel located in the southwest corner of the BGS Upper Ash Pond. The water in the channel routed along the south side of the gravel dike of the BGS Upper Ash Pond until it discharges into the southwest corner of the BGS Upper Ash Pond water body.

The water flows through the BGS Upper Ash Pond water body to the northeast towards a 24-inch wide precast concrete Parshall flume that discharges into a concrete catch basin. The water in the catch basin flows through a 15-inch diameter polyvinyl chloride pipe and discharges into the BGS Lower Pond. Instrumentation associated with the BGS Upper Ash Pond includes a flow meter that monitors the discharges. The discharge from the concrete catch basin enters the Lower Pond. The Lower Pond contains the facility's National Pollutant Discharge Elimination System (NPDES) Outfall 001. The water flows through the NPDES Outfall 001 hydraulic structure, which consists of cast in place weir box.

The total surface area of the BGS Upper Ash Pond is approximately 13.3 acres and has an embankment height of approximately 10 feet from the crest to the toe of the downstream slope. The elevation of the embankments is 531 feet, 3 feet lower than the 100 year flood elevation of the Mississippi River. The embankment is armored with cobble size stone on the crest and both outer and inner embankment slopes to prevent erosion of the



embankment during overtopping from extreme flood stage of the Mississippi River. The interior storage depth of the BGS Upper Ash Pond is approximately 7 feet. The volume of impounded CCR and water within the BGS Upper Ash Pond at normal operation water elevation is approximately 150,000 cubic yards.



3 HYDROLOGIC AND HYDRAULIC CAPACITY- §257.82(a)

This Report provides hydrologic and hydraulic capacity information for inflow design flood control systems which is intended to:

1. Adequately manage flow into each CCR unit during and following the peak discharge inflow of the specified design flood,
2. Adequately manage flow from each CCR unit during and following the peak discharge inflow of the specified design flood; and,
3. Handle discharge from the CCR unit in accordance with NPDES regulations §257.3-3.

3.1 Hazard Classification and Design Storm

Based on the 2016 Hazard Potential Classification analysis conducted by HHS, both the BGS Ash Seal Pond and BGS Main Ash Pond were classified as significant hazard potential due to the risk that pond contents could enter into the BGS Generating Station condenser discharge channel and from there into the Mississippi River. Based on the 2016 Hazard Potential Classification analysis conducted by HHS, both the BGS Economizer Pond and BGS Upper Ash Pond are classified as low hazard potential because if a release occurs, the contents would likely remain on BGS plant property.

The design storm for the BGS Ash Seal Pond and BGS Main Seal Pond is the 1,000 year return event SCS Type II 24 hour storm as designated in §257.82(a)(3)(ii). The design storm for the BGS Economizer Pond and the BGS Upper Ash Pond is the 100 year return event SCS Type II 24 hour storm as designated in §257.82(a)(3)(iii). The total rainfall for the two design events selected from the National Oceanographic and Atmospheric Administration's (NOAA) probabilistic map for the BGS Site coordinates is 10.3 inches for the 1,000 year event and 7.3 inches for the 100 year event, Appendix B.

Since surface water from the BGS Main Ash Pond must route through the BGS Upper Ash Pond, the Inflow Flood will be the 1,000 year event for the complete storm water system.



3.2 Hydrologic and Hydraulic Capacity Methods

3.2.1 BGS Ash Seal Pond

The 1,000 year SCS Type II storm of 10.3 inches accumulates in the storage pool of the BGS Ash Seal Pond without outflow. The total volume of the water is calculated by accumulating the rainfall on the watershed of 7.7 acres and storing the water in the BGS ash seal pond reservoir without discharge. The water elevation in the pond at the end of the design flood event is compared to the crest elevation of the embankment to determine the freeboard remaining at the end of the storm.

3.2.2 Storm Routing Through BGS Economizer Pond, BGS Main Ash Pond, and BGS Upper Ash Pond

The 1,000 year SCS Type II Storm was routed through the CCR units. The flow path is illustrated on Figure 2. The routing was completed using the program Hydraflow by Intelisolve¹. Hydraflow uses the unit hydrograph method to generate a Type II distributed rainfall distribution for each of the drainage area subsets in the watershed. Hydraflow then routes the unit hydrographs through the outlet structures of each BGS pond, storing water within the pond in accordance with the input reservoir capacity of the pond. The proportion of runoff to rainfall for each subunit of the drainage watershed is input based on the characteristics of the area.

The subareas of the flood control plan are shown in Figure 1 and include:

1. Storm water runoff from 8 acres of the plant site that is collected at the base of the BGS Economizer Pond at elevation 534 feet and pumped to the top of the BGS Economizer Pond at elevation 548 feet. Due to pump capacity, the flow is limited to a peak of 17.8 cubic feet per second (cfs).
2. Storm water runoff from 17 acres of the BGS Main Ash Pond.

¹Intelisolve. Pond Routing Software Hydraflow, 2002



3. Storm water runoff from 25 acres of the BGS Economizer Pond and BGS Upper Ash Pond combined.

4. In addition to the storm water flows identified above, a BGS process water flow of 1.0 cfs is added to the BGS Main Ash Pond and 1.5 cfs is added to the plant site water flow discharged to the BGS Economizer Pond, Appendix A.

3.3 Hydrologic and Hydraulic Capacity Input and Assumptions

This section identifies the input and assumptions for the hydrologic and hydraulic capacity calculations. The input for each drainage subarea is:

Sub-Area	Acreage	Curve Number (CN)	Slope (%)	Hydraulic Length (ft)
Plant Site	8	90	1.0	600
Main Ash Pond	17	85	1.5	1,250
Economizer Ditch	Open channel	Mannings n = 0.009	0	1,000
Economizer and Upper Ash Ponds	25	87	1.5	1,250

The CN for the plant site is typical of industrial sites with considerable paved areas. The CN for the ash pond areas reflects the ability to infiltrate into unsaturated CCR which is exposed and available to store infiltrated water early in the storm distribution.

Three reservoirs are part of the flood routing system and consist of the BGS Main Ash Pond, the BGS Economizer Ditch and Pond, and the BGS Upper Ash Pond. The outlet structures for each pond and the maximum storage capacity of the pond at maximum flow are:

Pond Reservoir	Total Storage (acre-feet)	Outlet Structure	Invert Elevation (feet)	Secondary Outlet Structure	Invert Elevation (feet)
BGS Main Ash Pond	11.0	2 – 15 inch CMP culverts	531.1	none	
BGS Economizer Pond	0.21	1 – 18 inch HDPE	544.0	1 – 12 foot wide rock filled weir	546.5



BGS Upper Ash Pond	51.8	Monitoring Flume + 15 inch PVC pipe	527.4	1 – 14 inch pipe with manual operated gate valve	529.0
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The details of the BGS Economizer Pond and BGS Upper Ash Pond outfall structures are shown in Appendix C. There are no available design drawings for the BGS Main Ash Pond outfall which is shown in the pictures contained in Appendix C.

The assumptions which impact the analysis of the flood routing are:

1. The outfall structures are not submerged on the tailwater and are therefore inlet restricted. This is a realistic assumption for the BGS Main Ash Pond and the BGS Economizer Pond. For the BGS Upper Ash Pond with the outlet invert of 523.4 feet, the tail could be submerged if the Mississippi River is in flood stage above approximately 525 feet. This is a reasonable assumption since Mississippi River flood stage and design flood are unlikely to be coincident.
2. The emergency overflow pipe on the BGS Upper Ash Pond has a manually operated valve. It is assumed that this valve is closed and all of the flood flow must pass through the flume and 15-inch diameter discharge pipe.
3. The rainfall runoff on the Plant Area (Figure 2) is stored on the plant area when the pump capacity of the lift pumps is exceeded and the volume is recovered as the pumps catch-up after the precipitation event. This volume could overflow directly to the BGS Upper Ash Pond reducing the duration of the maximum pumping capacity event.
4. To be conservative, no exfiltration of water through the bottom of the ponds is considered in the analysis. Since the natural soil under the ponds is clay the assumption likely holds for all ponds except for the BGS Economizer Pond where a substantial unsaturated zone is available to accept exfiltrate water during the rainfall event.



5. The beginning water elevation condition in the ponds at the start of the inflow flood is the normal operating elevations, or in the case of the BGS Ash Seal Pond, no water.



4 INFLOW DESIGN FLOOD CONTROL SYSTEM PLAN

The inflow flood caused by a 1,000 year return event SCS Type II storm distribution is routed through the CCR pond system at BGS. The routing includes the BGS Ash Seal Pond that operates as a no discharge pond and the remaining ash ponds that discharge through NPDES Outfall 001.

4.1 BGS Ash Seal Pond Plan

The 7.7 acres of the BGS Ash Seal Pond will accumulate 6.6 acre-feet of water during the design storm. Without discharging water, the final water elevation in the pond will be 533.4 feet with a freeboard of 0.6 feet. The results of the calculation of capacity are presented in Appendix D.

The area of the BGS Ash Seal pond is located on the natural levee deposits of the Mississippi River and unlike the other ash ponds located west of the levee, Figure 1, has a sandy natural soil under the pond. The actual water elevation in the pond is likely to be lower due to exfiltration of water during the storm event.

4.2 BGS Ash Ponds Draining to BGS Upper Ash Pond Outfall

The 50 acres of general plant area, BGS Main Ash Pond, BGS Economizer Pond, and BGS Upper Ash Pond will discharge a maximum flow of 9.4 cfs at the BGS Upper Ash Pond outfall during the inflow flood routing. Between the BGS Main Ash Pond and the BGS Upper Ash Pond, 36 acre feet of water will be stored above the normal operating elevation of the two ponds at maximum discharge. The storage will occur with a water elevation of 533.4 feet in the BGS Main Ash Pond (freeboard 0.6 foot) and 530.3 feet in the BGS Upper Ash Pond (freeboard 0.7 feet). The hydrographs, reservoir and outlet details are presented in Appendix E.

The flow from the BGS Economizer Pond includes the storm water from the plant area and the process water flow of 1.5 cfs. Based on both 4,000 gpm pumps in the combined storm water and process water sump operating, the maximum flow to the storm water



and process water treatment channel on top of the BGS Economizer Pond is 19.3 cfs. This flow will occur for approximately 1 hour during the peak of the inflow flood event and will cause the capacity of the existing 18-inch HDPE outfall pipe from the BGS Economizer Pond to be exceeded. In order to route the entirety of the peak inflow flood event the previously existing 12-inch diameter steel pipe was removed and replaced with a 12-foot wide 18-inch deep rock filled weir. The rock filled weir, constructed in June 2017, will route the excess flow through the riprap and along the downstream slope of the embankment into the BGS Upper Ash Pond.

Approximately 0.22 acre feet of water will be stored above the normal operating elevation of the BGS Economizer Pond at maximum discharge. The storage will occur with a water elevation of 547.5 feet (freeboard 0.5 feet). The hydrographs, reservoir and outlet details are presented in Appendix E.



5 QUALIFIED PROFESSIONAL ENGINEER CERTIFICATION

To meet the requirements of 40 CFR 257.82(c)(5), I Mark W. Loerop hereby certify that I am a licensed professional engineer in the State of Iowa; and that, to the best of my knowledge, all information contained in this document is correct and the document was prepared in compliance with all applicable requirements in 40 CFR 257.82.

The initial Inflow Flood Control Plan, dated August 25, 2016, was prepared and certified by a qualified professional engineer. The next periodic review of this Inflow Flood Control Plan is required to be complete by August 2021.

This certification is for Revision 1 of the Inflow Flood Control Plan, which includes the following amendments:

- Revisions to the Inflow Flood Control hydraulic analysis due to the modifications of the hydraulic structures associated with the discharge from the BGS Economizer Pond and the storm water and process water treatment channel into the BGS Upper Ash Pond.



By: 

Name: Mark Loerop

Date: July 26, 2017



FIGURES

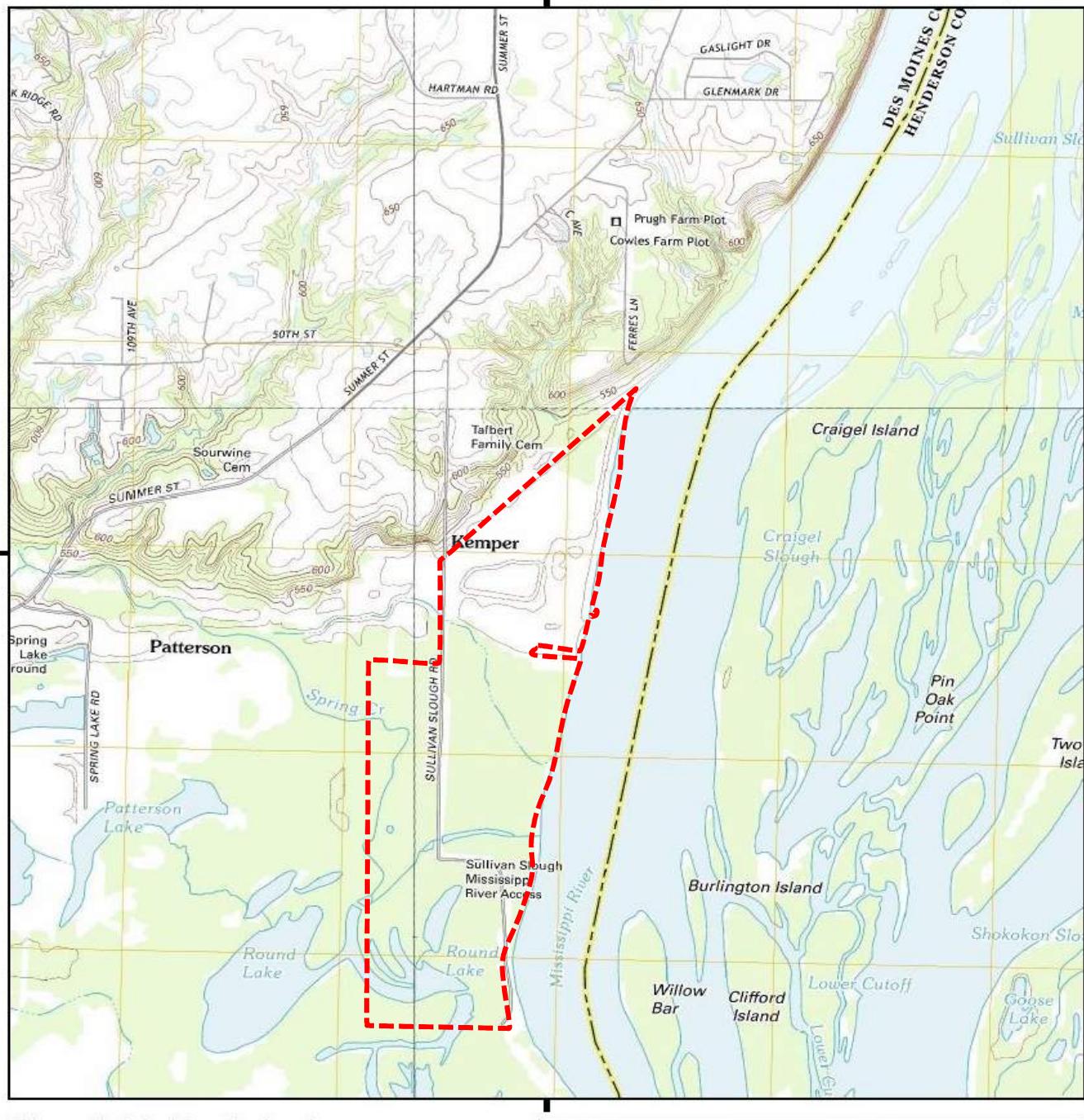
Alliant Energy
Interstate Power and Light Company
Burlington Generating Station
Burlington, Iowa

Inflow Design Flood Control System Plan

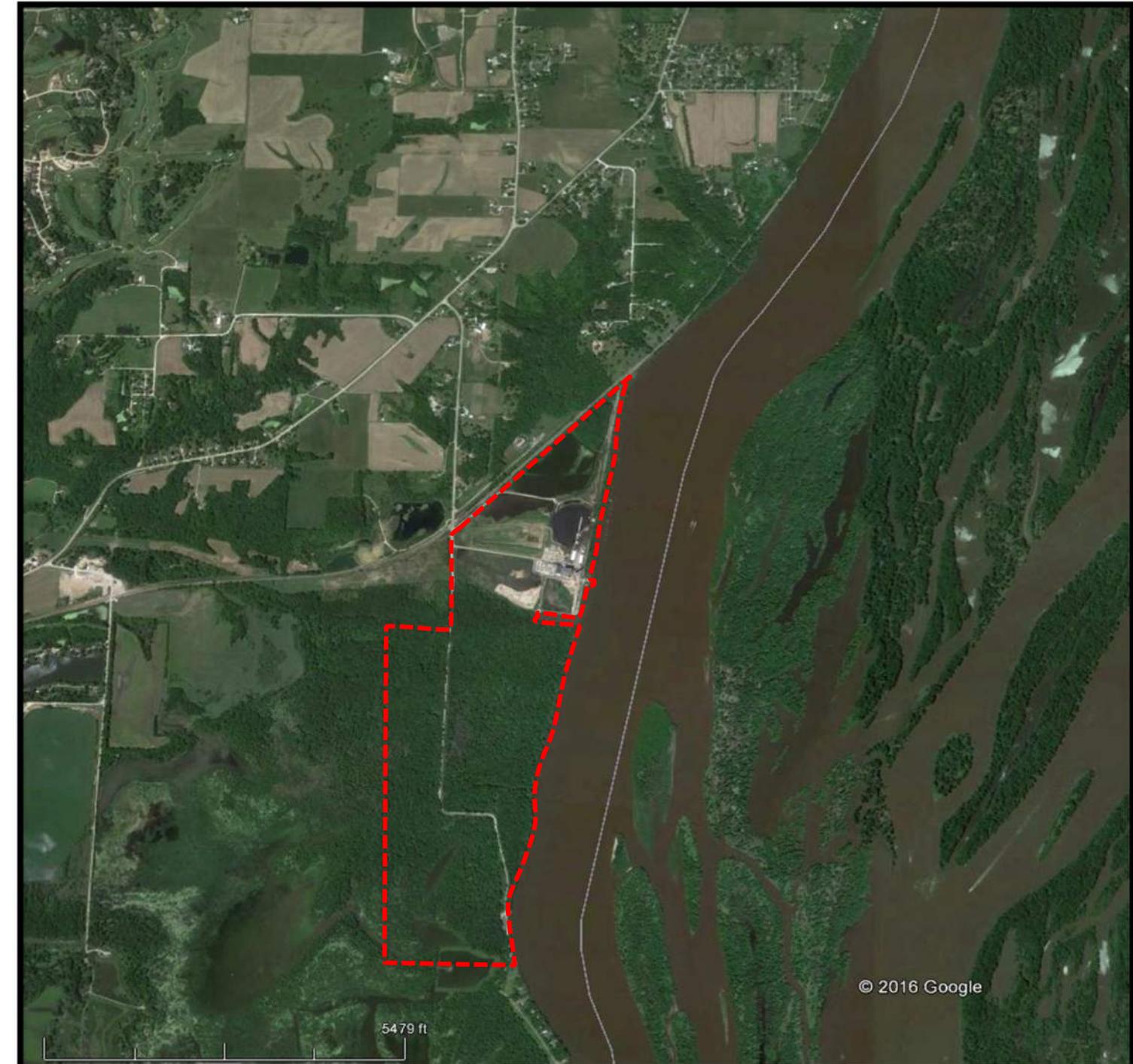


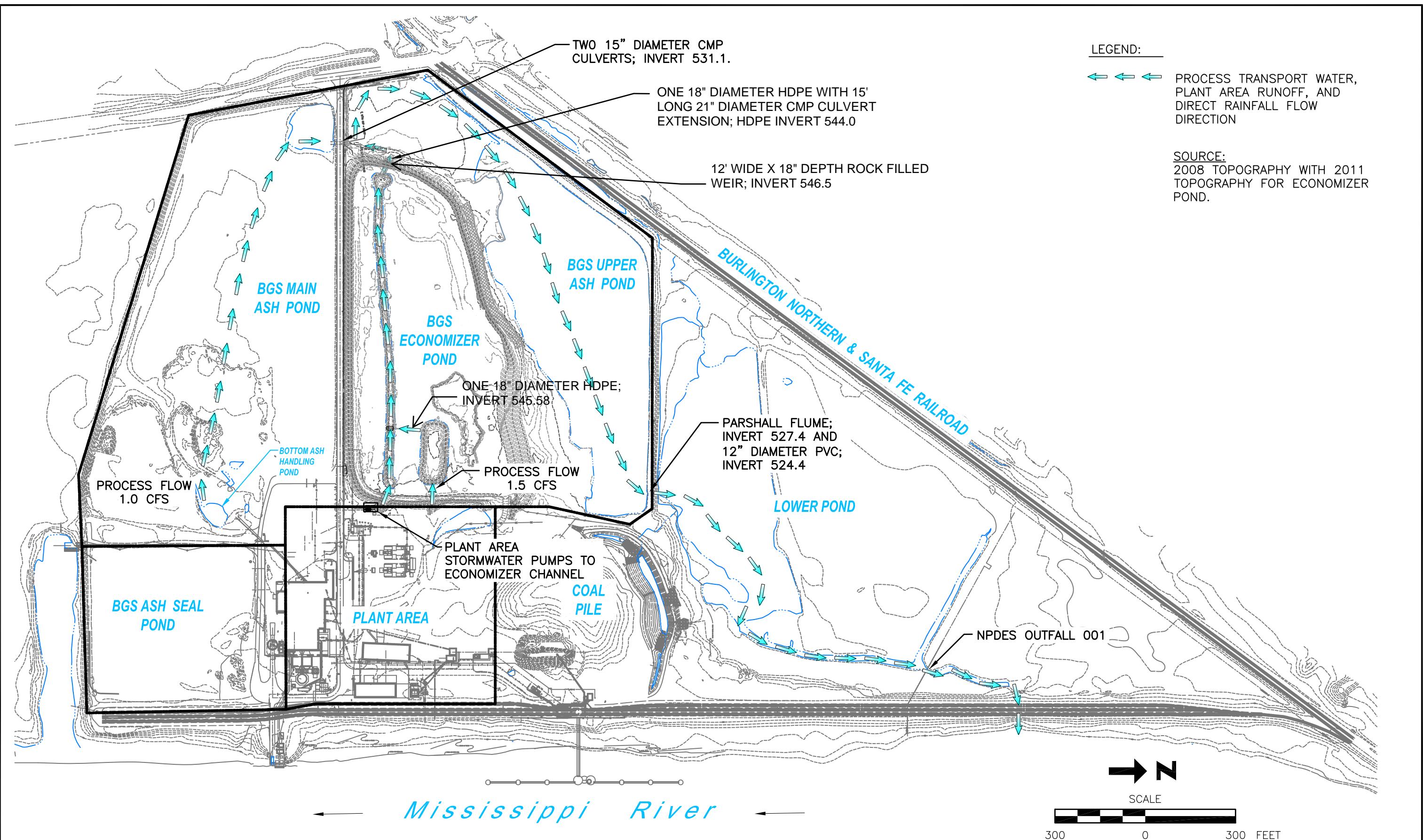
Historical Topo Map

2012, 2013



Historical Aerial Photo 6/12/2014





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CLIENT / LOCATION

ALLIANT ENERGY
BURLINGTON GENERATING STATION
BURLINGTON, IOWA

DRAWING DESCRIPTION

INFLOW FLOOD CONTROL SITE PLAN

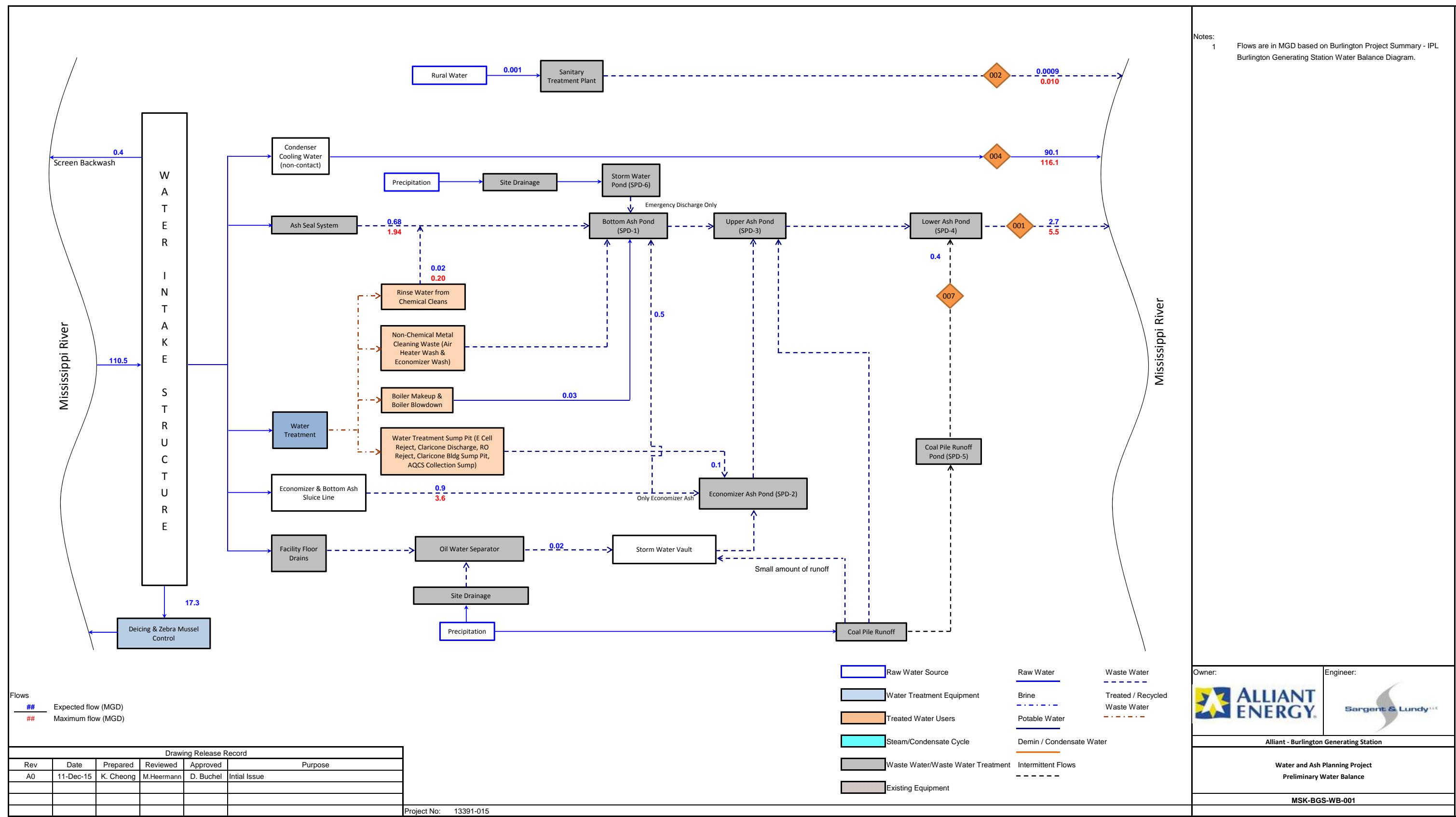
B 154.018.012.001
IT. FIGURE 1
VG. 154.018.012.001-D1

APPENDIX A – BGS Water Balance Chart

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Burlington, Iowa

Inflow Design Flood Control System Plan





APPENDIX B – NOAA Storm Frequency

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Burlington Generating Station
Burlington, Iowa

Inflow Design Flood Control System Plan





NOAA Atlas 14, Volume 8, Version 2
Location name: Burlington, Iowa, US*
Latitude: 40.7426°, Longitude: -91.1201°
Elevation: 545 ft*
* source: Google Maps



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerials](#)

PF tabular

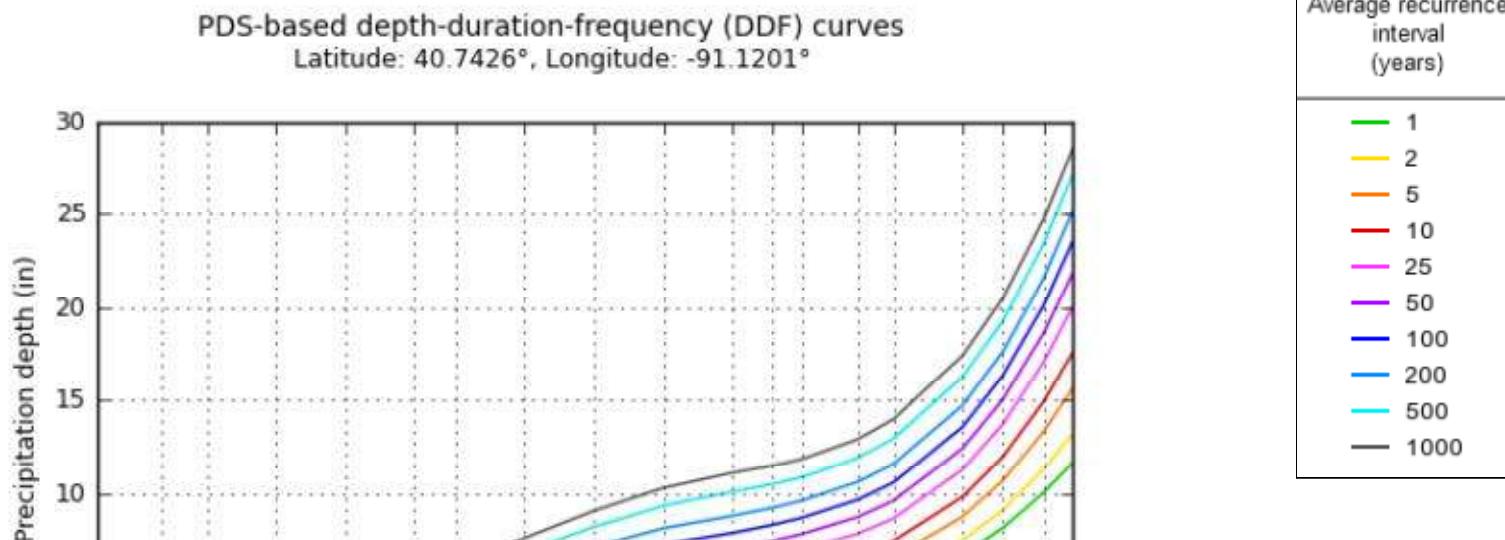
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.384 (0.326–0.458)	0.449 (0.381–0.537)	0.555 (0.470–0.664)	0.642 (0.541–0.770)	0.760 (0.620–0.924)	0.849 (0.680–1.04)	0.937 (0.728–1.16)	1.03 (0.769–1.29)	1.14 (0.826–1.46)	1.23 (0.869–1.59)
10-min	0.562 (0.477–0.671)	0.658 (0.559–0.786)	0.813 (0.688–0.973)	0.940 (0.792–1.13)	1.11 (0.908–1.35)	1.24 (0.995–1.52)	1.37 (1.07–1.71)	1.50 (1.13–1.89)	1.67 (1.21–2.14)	1.79 (1.27–2.32)
15-min	0.685 (0.582–0.818)	0.802 (0.681–0.958)	0.991 (0.840–1.19)	1.15 (0.966–1.38)	1.36 (1.11–1.65)	1.52 (1.21–1.86)	1.67 (1.30–2.08)	1.83 (1.37–2.31)	2.04 (1.48–2.61)	2.19 (1.55–2.83)
30-min	0.952 (0.810–1.14)	1.12 (0.953–1.34)	1.39 (1.18–1.67)	1.62 (1.36–1.94)	1.91 (1.56–2.32)	2.14 (1.71–2.62)	2.36 (1.83–2.92)	2.57 (1.93–3.24)	2.85 (2.07–3.65)	3.06 (2.17–3.96)
60-min	1.23 (1.05–1.47)	1.43 (1.22–1.71)	1.77 (1.50–2.12)	2.05 (1.73–2.46)	2.44 (2.00–2.98)	2.75 (2.20–3.38)	3.06 (2.38–3.81)	3.38 (2.54–4.27)	3.81 (2.77–4.89)	4.14 (2.94–5.36)
2-hr	1.51 (1.30–1.79)	1.75 (1.49–2.07)	2.14 (1.83–2.54)	2.48 (2.11–2.95)	2.97 (2.46–3.61)	3.36 (2.72–4.11)	3.76 (2.96–4.66)	4.19 (3.18–5.26)	4.77 (3.50–6.09)	5.22 (3.74–6.71)
3-hr	1.69 (1.45–1.99)	1.93 (1.66–2.28)	2.36 (2.02–2.78)	2.74 (2.33–3.23)	3.29 (2.75–4.00)	3.75 (3.06–4.58)	4.24 (3.35–5.24)	4.75 (3.63–5.97)	5.48 (4.04–6.98)	6.06 (4.36–7.74)
6-hr	1.99 (1.72–2.32)	2.28 (1.98–2.66)	2.80 (2.42–3.28)	3.27 (2.81–3.83)	3.97 (3.34–4.79)	4.55 (3.75–5.52)	5.18 (4.13–6.36)	5.85 (4.51–7.29)	6.79 (5.06–8.59)	7.55 (5.48–9.58)
12-hr	2.29 (2.00–2.65)	2.68 (2.34–3.10)	3.35 (2.92–3.89)	3.95 (3.42–4.58)	4.81 (4.07–5.75)	5.52 (4.57–6.63)	6.27 (5.04–7.62)	7.06 (5.47–8.71)	8.16 (6.12–10.2)	9.03 (6.60–11.4)
24-hr	2.66 (2.34–3.04)	3.11 (2.74–3.57)	3.90 (3.42–4.48)	4.58 (4.00–5.27)	5.57 (4.75–6.58)	6.37 (5.31–7.57)	7.21 (5.83–8.68)	8.09 (6.32–9.88)	9.30 (7.03–11.5)	10.3 (7.57–12.8)
2-day	3.11 (2.76–3.53)	3.58 (3.17–4.06)	4.38 (3.87–4.98)	5.08 (4.47–5.80)	6.11 (5.25–7.16)	6.95 (5.85–8.19)	7.83 (6.40–9.35)	8.76 (6.91–10.6)	10.1 (7.67–12.4)	11.1 (8.24–13.7)
3-day	3.38	3.89	4.74	5.48	6.54	7.39	8.27	9.20	10.5	11.5

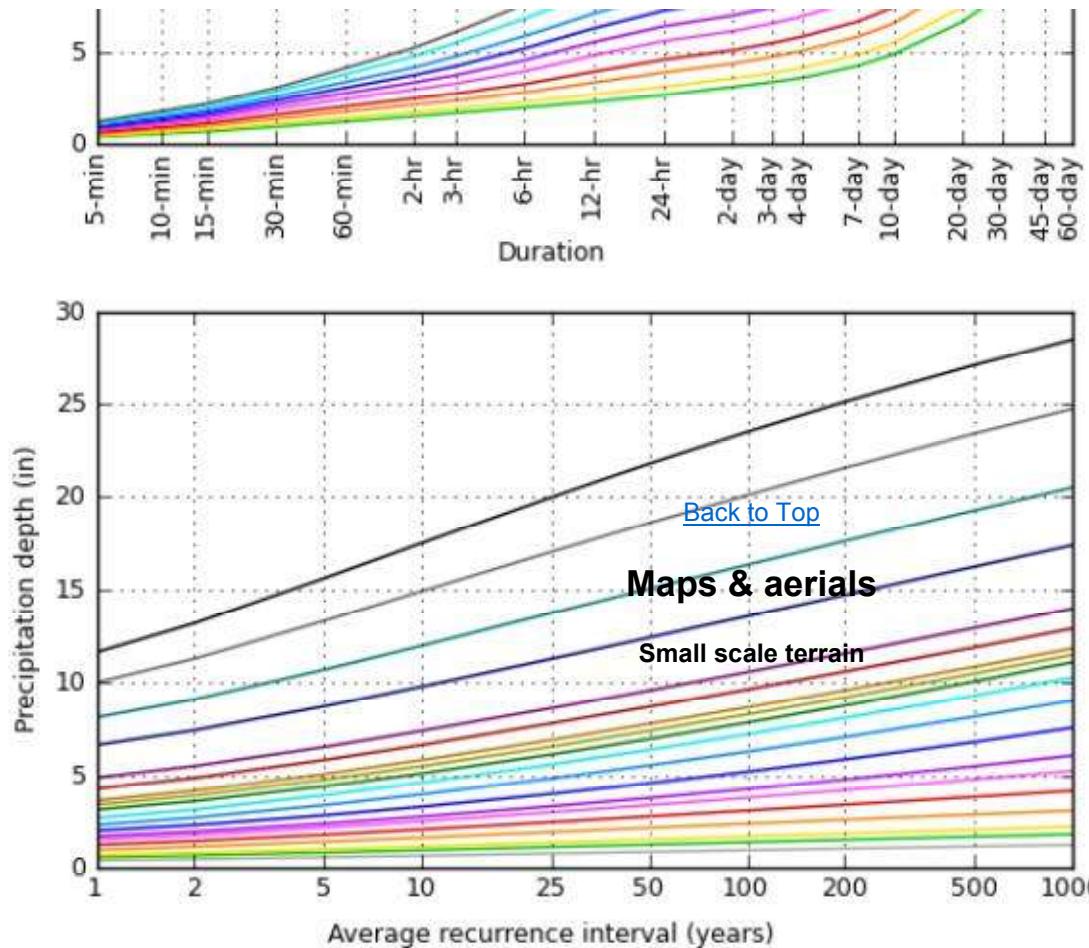
	(3.01–3.82)	(3.46–4.39)	(4.20–5.36)	(4.83–6.21)	(5.63–7.59)	(6.24–8.64)	(6.78–9.81)	(7.28–11.1)	(8.02–12.8)	(8.58–14.1)
4-day	3.61 (3.23–4.06)	4.15 (3.70–4.67)	5.04 (4.49–5.69)	5.81 (5.15–6.57)	6.90 (5.96–7.97)	7.77 (6.57–9.03)	8.66 (7.12–10.2)	9.58 (7.60–11.5)	10.8 (8.32–13.2)	11.8 (8.87–14.5)
7-day	4.26 (3.83–4.75)	4.84 (4.35–5.41)	5.81 (5.20–6.50)	6.63 (5.91–7.44)	7.79 (6.76–8.91)	8.70 (7.40–10.0)	9.63 (7.96–11.3)	10.6 (8.45–12.6)	11.9 (9.18–14.4)	12.9 (9.73–15.7)
10-day	4.86 (4.39–5.40)	5.48 (4.95–6.09)	6.52 (5.86–7.25)	7.39 (6.61–8.25)	8.61 (7.51–9.81)	9.57 (8.18–11.0)	10.6 (8.76–12.3)	11.6 (9.27–13.7)	12.9 (10.0–15.5)	14.0 (10.6–17.0)
20-day	6.63 (6.03–7.29)	7.42 (6.75–8.17)	8.71 (7.90–9.61)	9.79 (8.83–10.8)	11.3 (9.88–12.7)	12.4 (10.7–14.1)	13.5 (11.3–15.6)	14.7 (11.9–17.2)	16.2 (12.7–19.3)	17.4 (13.3–20.9)
30-day	8.11 (7.42–8.88)	9.10 (8.31–9.96)	10.7 (9.73–11.7)	12.0 (10.9–13.2)	13.7 (12.1–15.3)	15.0 (13.0–16.9)	16.3 (13.7–18.7)	17.6 (14.3–20.5)	19.3 (15.1–22.8)	20.5 (15.8–24.6)
45-day	10.0 (9.19–10.9)	11.3 (10.3–12.3)	13.3 (12.2–14.5)	14.9 (13.6–16.3)	17.0 (15.0–18.8)	18.6 (16.1–20.8)	20.1 (16.9–22.8)	21.6 (17.6–24.9)	23.4 (18.5–27.5)	24.7 (19.1–29.5)
60-day	11.6 (10.7–12.6)	13.2 (12.1–14.3)	15.6 (14.3–16.9)	17.5 (16.0–19.1)	20.0 (17.7–22.0)	21.8 (18.9–24.2)	23.5 (19.8–26.5)	25.1 (20.5–28.8)	27.1 (21.4–31.7)	28.5 (22.1–33.8)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.
Please refer to NOAA Atlas 14 document for more information.

[Back to Top](#)

PF graphical





NOAA Atlas 14, Volume 8, Version 2

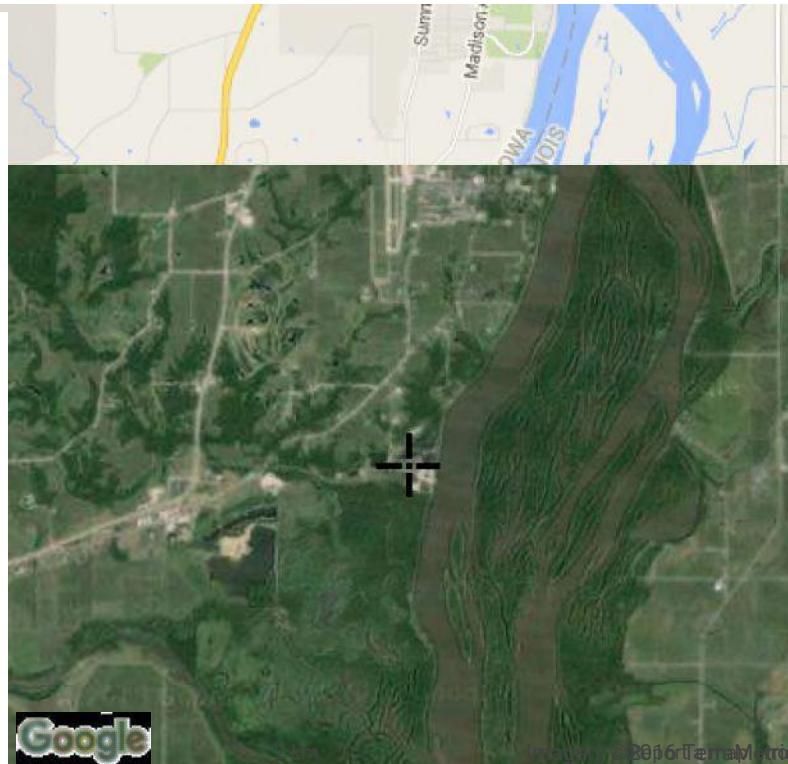
Created (GMT): Fri Apr 1 13:24:39 2016





Large scale map





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[US Department of Commerce](#)
[National Oceanic and Atmospheric Administration](#)
[National Weather Service](#)
[National Water Center](#)
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

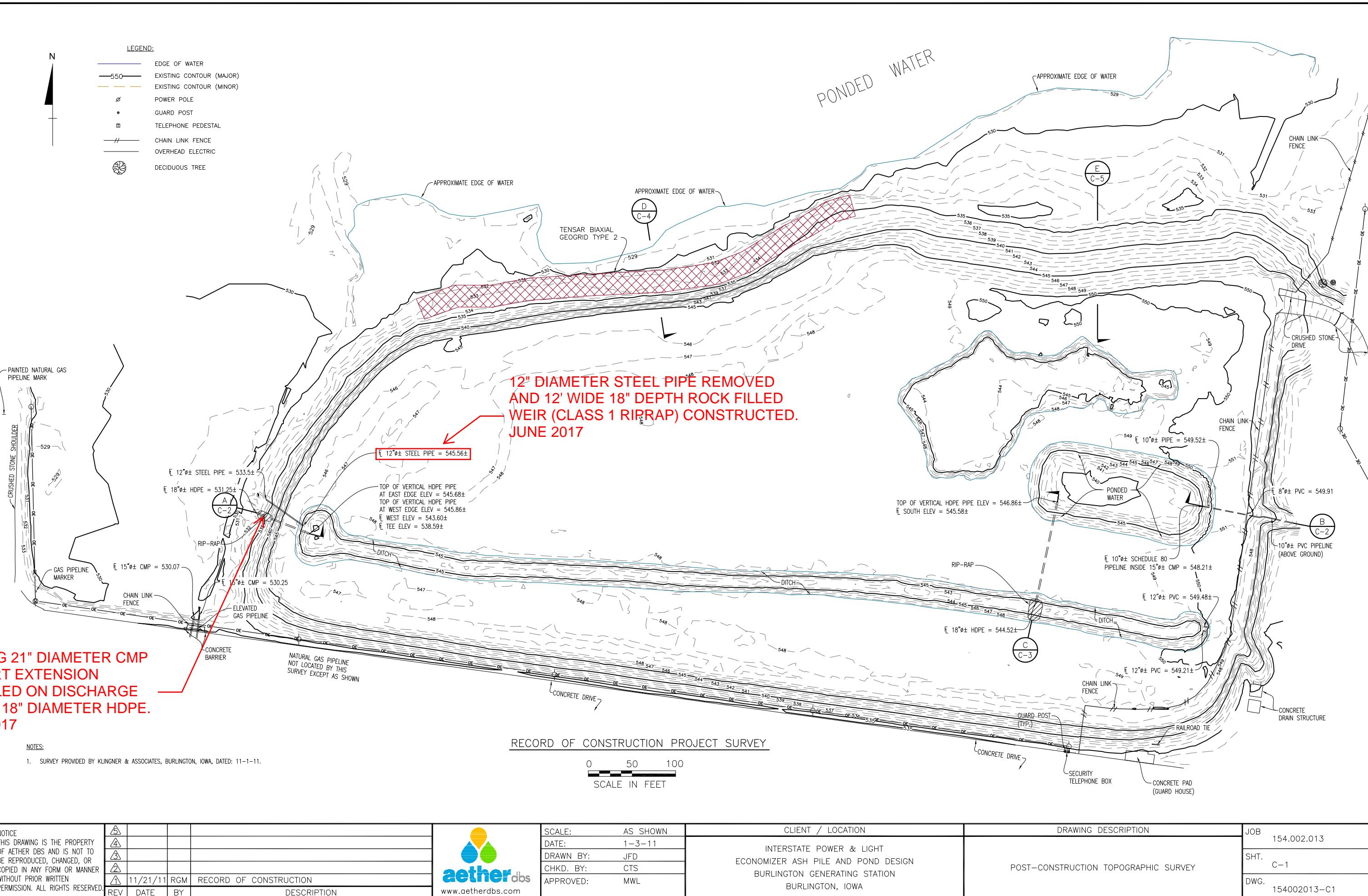
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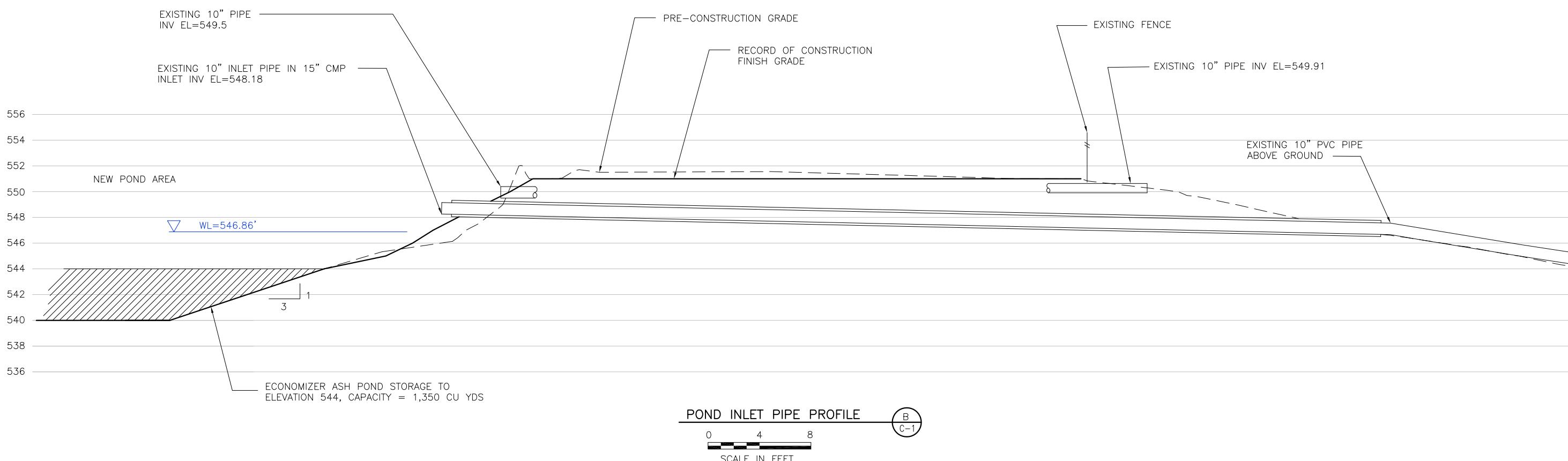
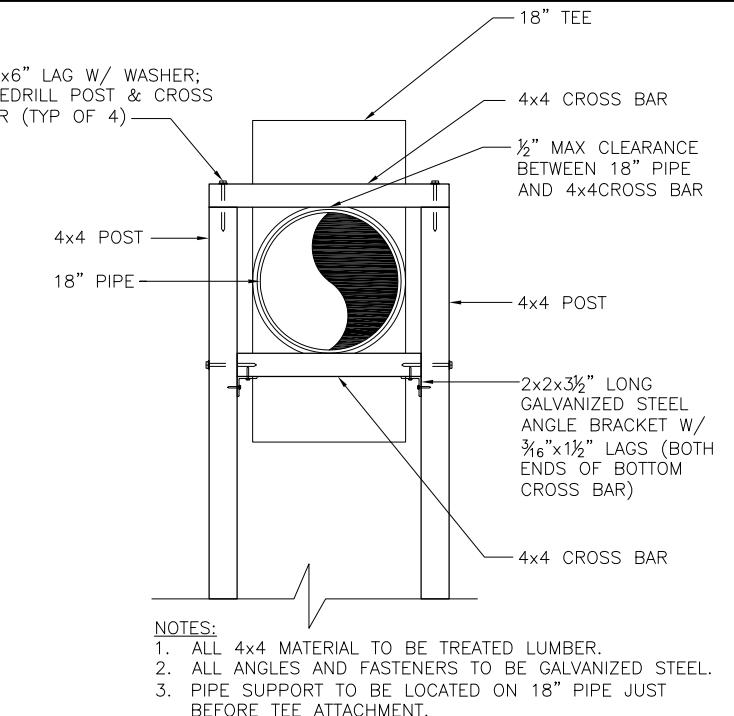
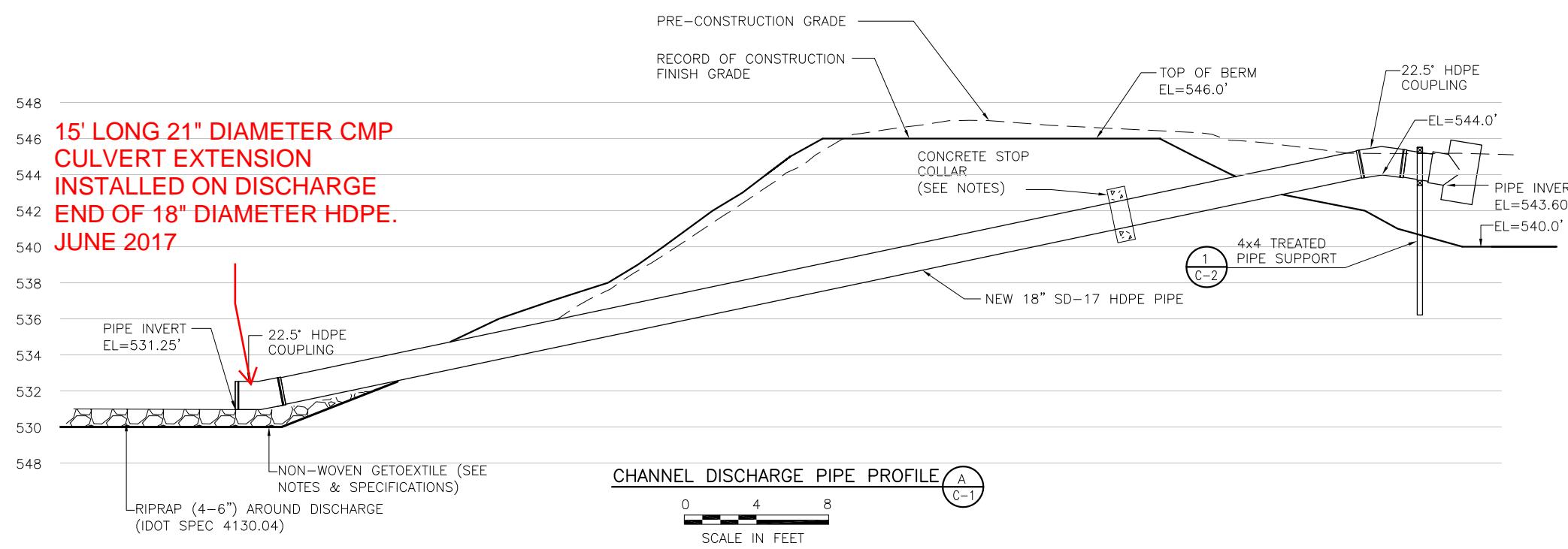
APPENDIX C – Outfall Drawings

Alliant Energy
Interstate Power and Light Company
Burlington Generating Station
Burlington, Iowa

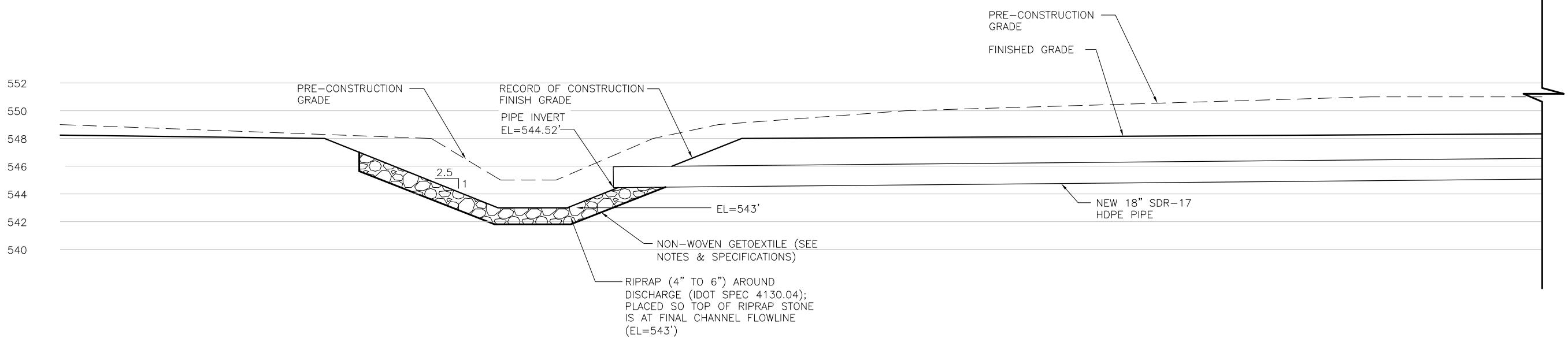
Inflow Design Flood Control System Plan



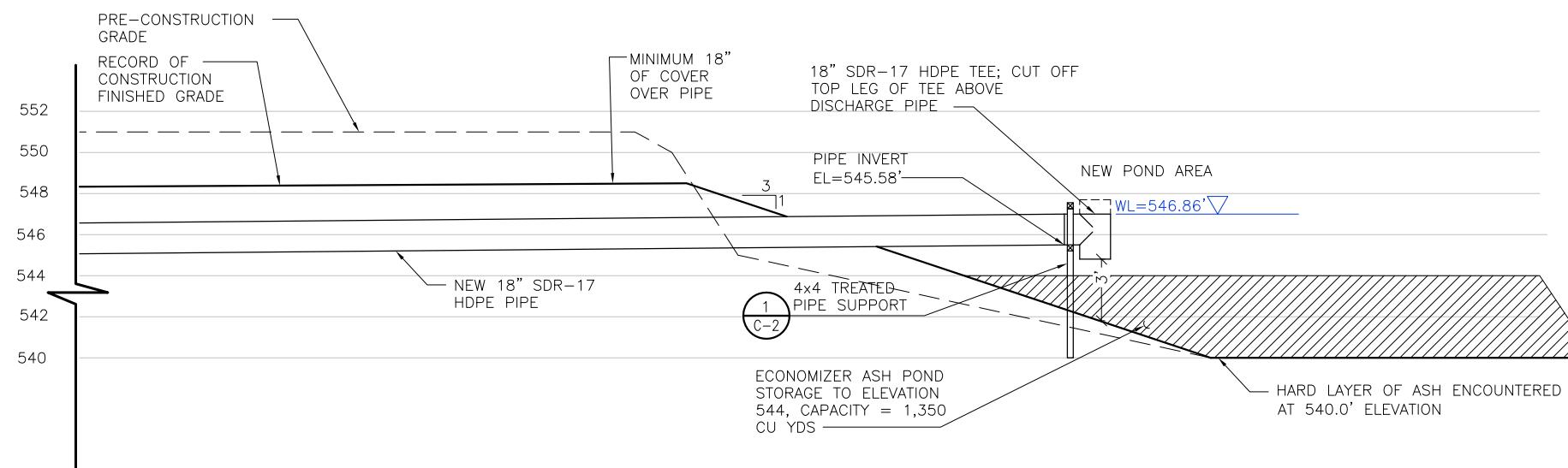




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11/21/11	RGM	RECORD OF CONSTRUCTION	
8/22/11	MWL	REVISED POND LOCATION	
REV	DATE	BY	DESCRIPTION



POND DISCHARGE PIPE PROFILE C
C-1
0 4 8
SCALE IN FEET



POND DISCHARGE PIPE PROFILE (CONT) C
C-1
0 4 8
SCALE IN FEET

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11/21/11 RGM RECORD OF CONSTRUCTION
8/22/11 MWL REVISED POND LOCATION
REV DATE BY DESCRIPTION



SCALE: AS SHOWN
DATE: 8-22-11
DRAWN BY: JFD
CHKD. BY: CTS
APPROVED: MWL

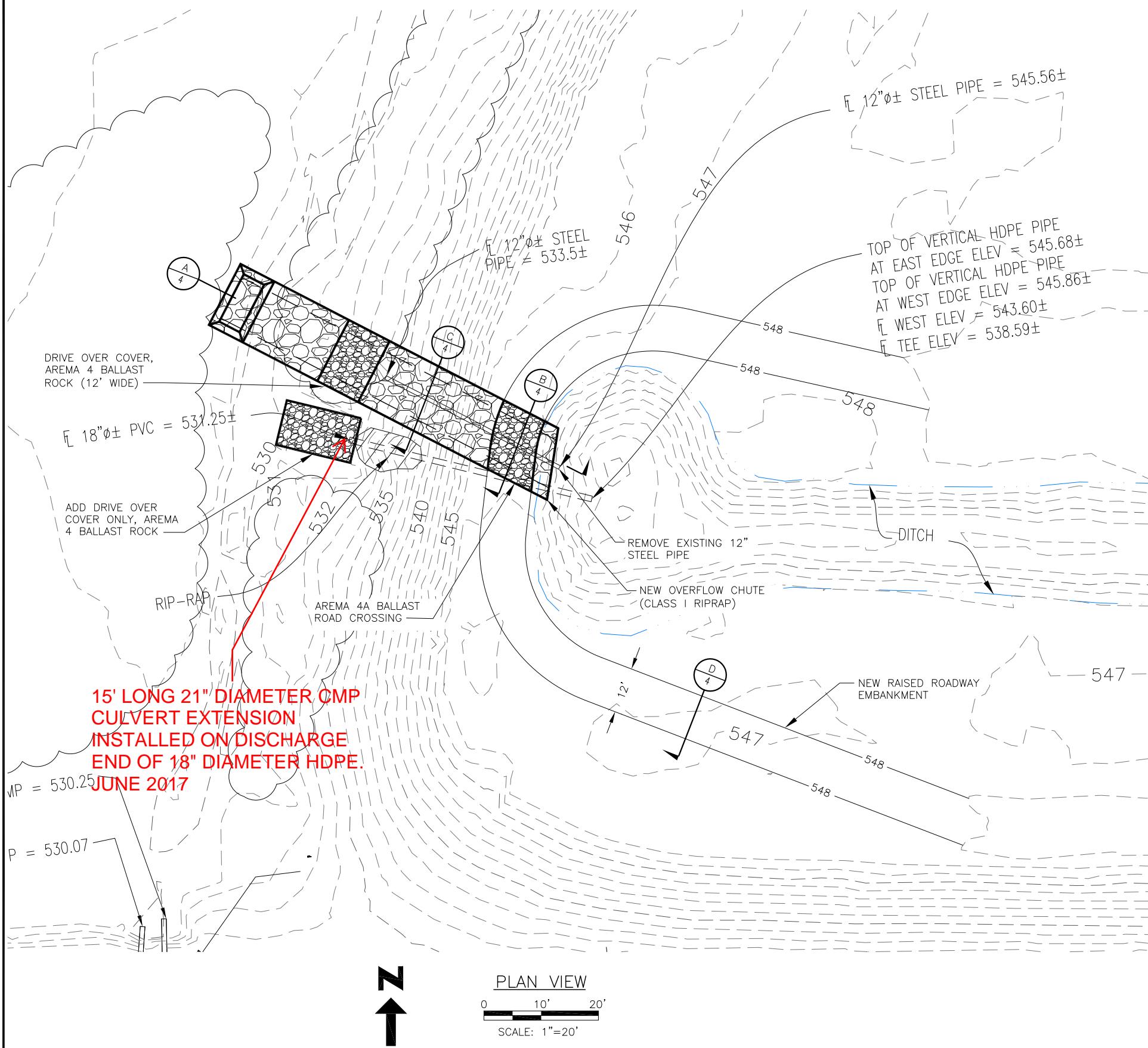
CLIENT / LOCATION
INTERSTATE POWER & LIGHT
ECONOMIZER ASH PILE AND POND DESIGN
BURLINGTON GENERATING STATION
BURLINGTON, IOWA

DRAWING DESCRIPTION
GRADING SECTION C

JOB 154.002.013
SHT. C-3
DWG. 154002013-C3

SOURCE:
2008 TOPOGRAPHY WITH 2011
TOPOGRAPHY FOR ECONOMIZER POND,
KLINGER & ASSOCIATES.

- INSTRUCTIONS:
1. RETAIN EXISTING HDPE NORMAL FLOW DISCHARGE PIPE.
 2. REMOVE EXISTING FLOOD FLOW OVERFLOW PIPE AND REPLACE WITH ROCK FILLED OVERFLOW CHUTE.
 3. PROVIDE SURFACE OF BALLAST ROCK ON BOTH CREST AND TOE OF NEW ROCK CHUTE AND ON TOE OF EXISTING RIPRAP OF HDPE DISCHARGE PIPE.
 4. RAISE CREST OF EMBANKMENT TO ELEVATION 548 AT DISCHARGE LOCATION.



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REV DATE BY

DESCRIPTION

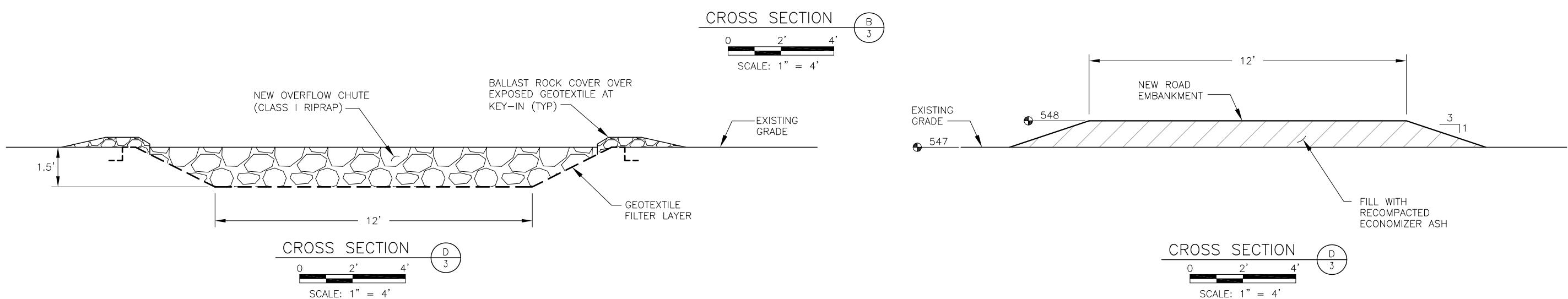
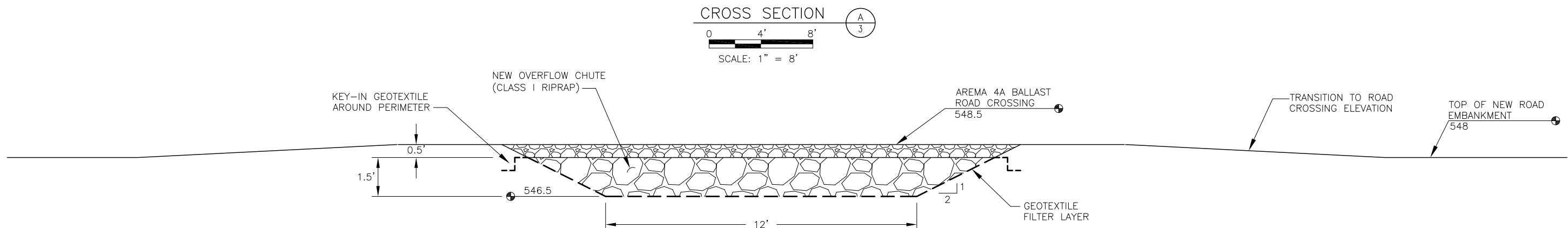
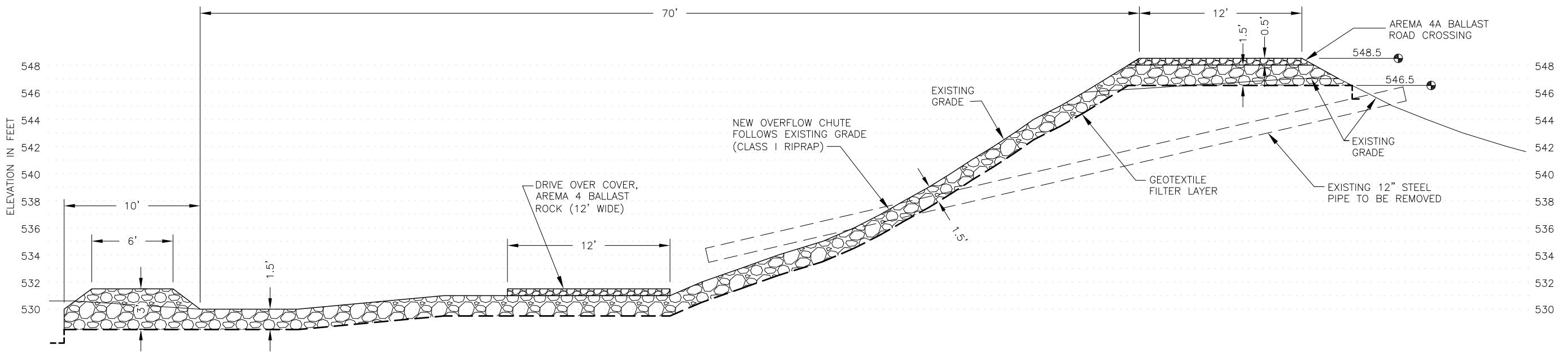


SCALE:	AS SHOWN
DATE:	7-28-16
DRAWN BY:	JFD
CHKD BY:	TJH
APRVD BY:	RAS

CLIENT / LOCATION
ALLIANT ENERGY
ASH SEAL POND EMBANKMENT ARMORING AND
ECONOMIZER ASH POND OVERFLOW DESIGN
BURLINGTON GENERATING STATION
BURLINGTON, IOWA

DRAWING DESCRIPTION
ECONOMIZER POND
OUTLET STRUCTURES
NEW OVERFLOW CHUTE

JOB 154.002.014
SHT. 3
DWG. 154.002.014.D3



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SCALE:	AS SH
DATE:	8-1-
DRAWN BY:	JFD
CHKD BY:	TJH
APRVD BY:	RAS

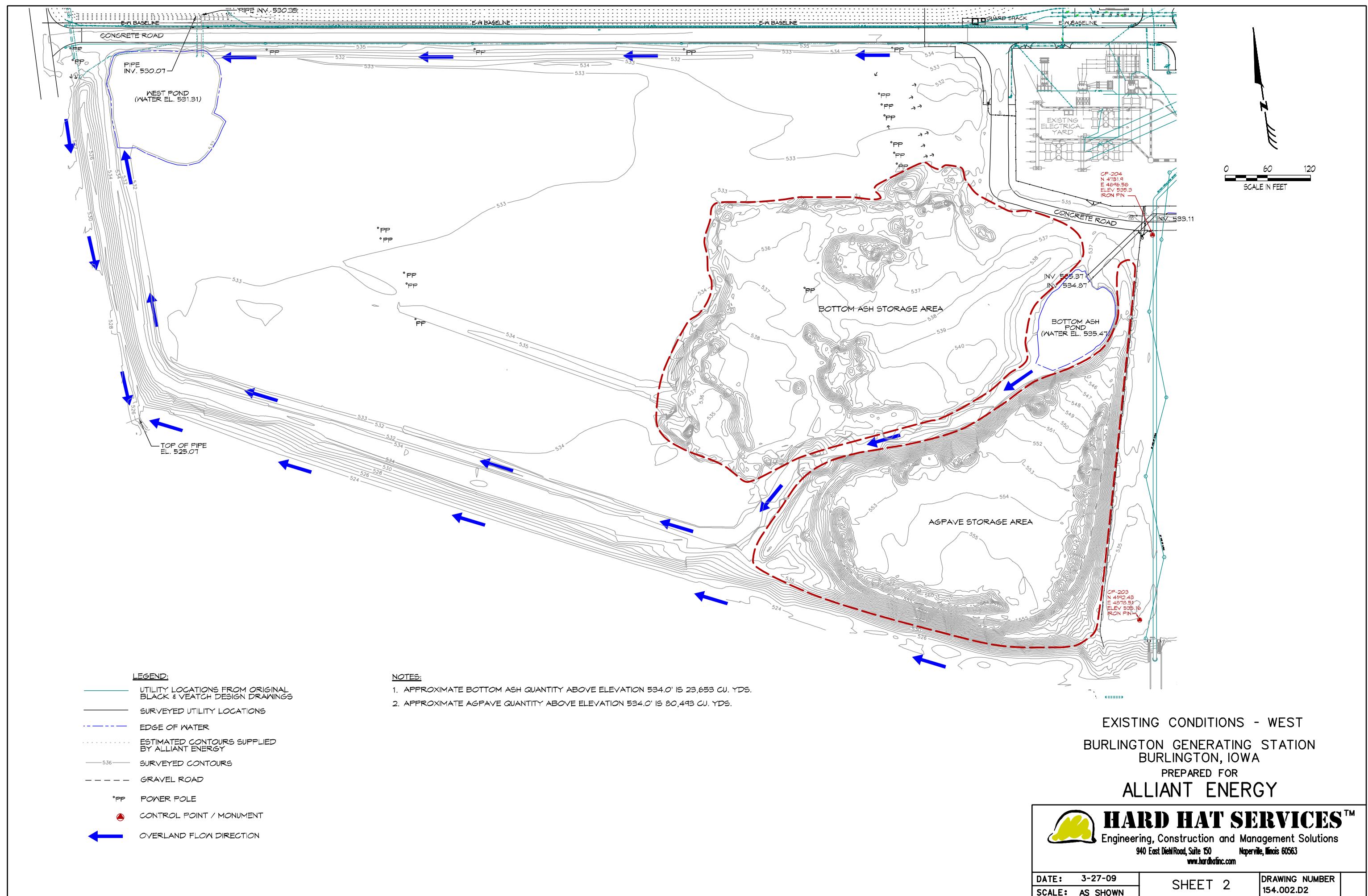
CLIENT / LOCATION
ALLIANT ENERGY
ASH SEAL POND EMBANKMENT ARMORING AND
ECONOMIZER ASH POND OVERFLOW DESIGN
BURLINGTON GENERATING STATION
BURLINGTON, IOWA

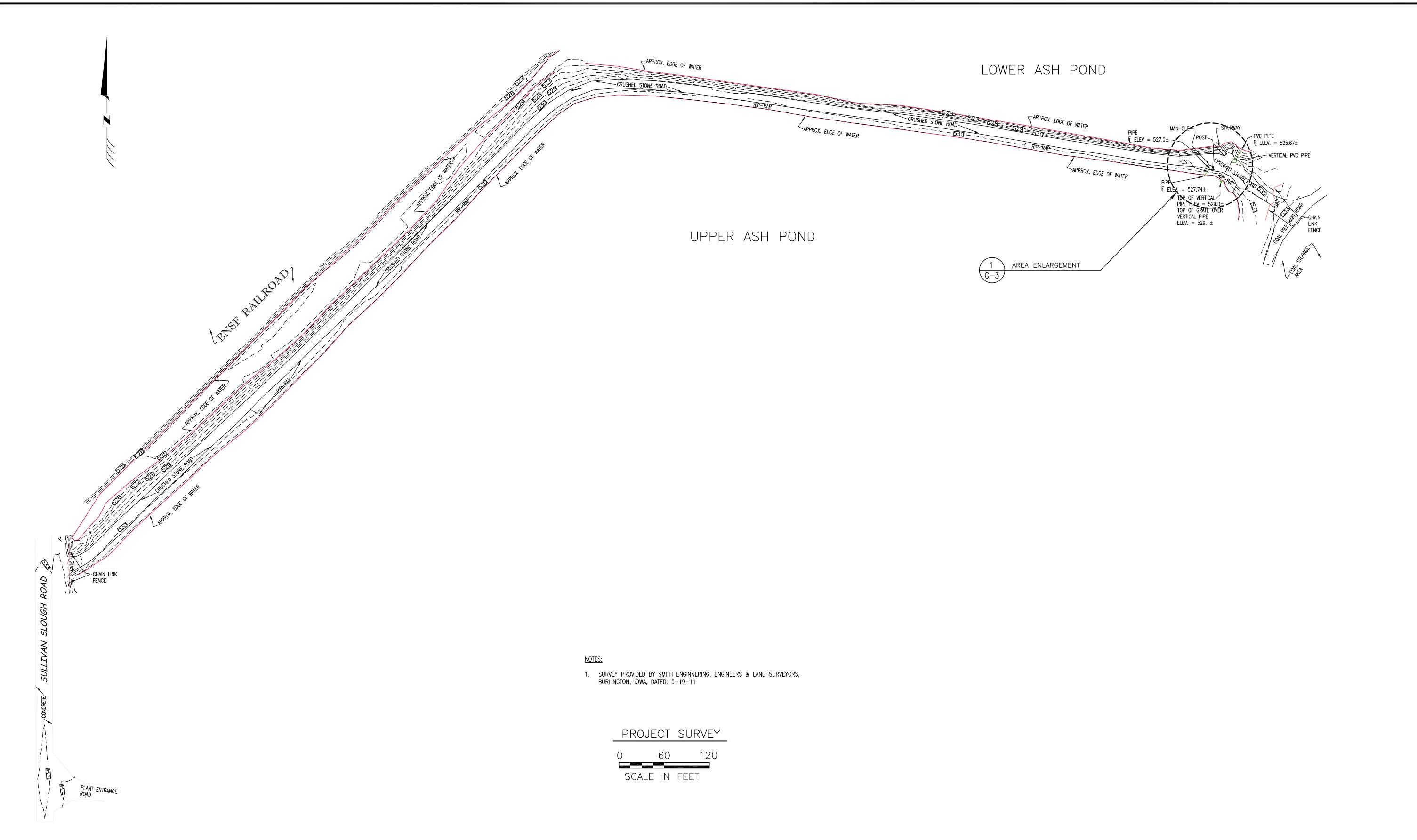
DRAWING DESCRIPTION

ECONOMIZER POND
OUTLET STRUCTURES

CROSS SECTIONS A - D

JOB	154.002.014
SHT.	4
DWG.	154.002.014.D3





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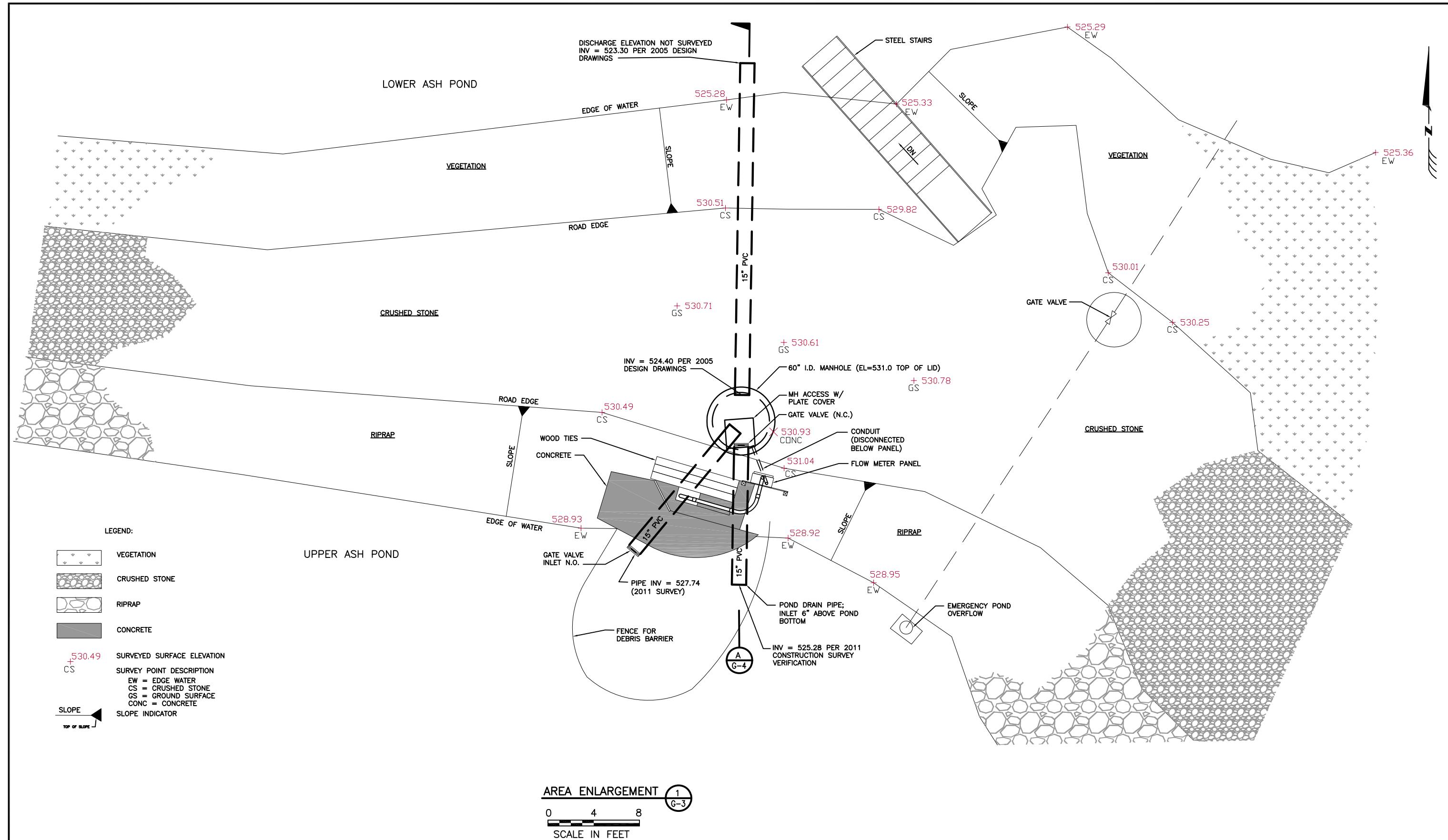


SCALE: AS SHOWN
DATE: 9-19-11
DRAWN BY: JFD
CHKD. BY:
APPROVED:

CLIENT / LOCATION
INTERSTATE POWER & LIGHT
UPPER ASH POND OUTFALL CONSTRUCTION AS-BUILTS
BURLINGTON GENERATING STATION
BURLINGTON, IA

DRAWING DESCRIPTION
PROJECT AREA SURVEY

JOB 154.002.010.001
SHT. G-2
DWG. 154002010-G2



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						DATE: 9-19-11	INTERSTATE POWER & LIGHT UPPER ASH POND OUTFALL CONSTRUCTION AS-BUILTS BURLINGTON GENERATING STATION BURLINGTON, IA	PROJECT AREA ENLARGEMENT	SHT. G-3
						DRAWN BY: JFD			DWG. 154002010-G3
						CHKD. BY:			
						APPROVED:			
REV	DATE	BY	DESCRIPTION						

APPENDIX D – BGS Ash Seal Pond Analysis

Alliant Energy
Interstate Power and Light Company
Burlington Generating Station
Burlington, Iowa

Inflow Design Flood Control System Plan





SHEET NO. 1 OF 1

PROJECT NO.

DATE APRIL 24, 2016

BY T-TH CKD MWL

ASH SEAL POND
INFLOW FLOOD ANALYSIS

1

2

3

4

PONDED AREA = 5.7 ACRES



5

ADDITIONAL WATERSHED = 2.0 ACRES



6

TOTAL = 7.7 ACRES



7

8

9

1000 YEAR RETURN STORM = 10.3 INCHES



10

TOTAL STORED VOLUME (NO EXCITRATION)

11

12

13

VOLUME = 10.3 INCHES / 12 IN/FT × 7.7 ACRES



14

= 6.6 ACRE/FEET



15

= 287,500 FEET³

16

17

18

* BOTTOM ELEVATION IN SOUTHWEST CORNER 531 FT

19

20

21

AREA @ 531 36,000 D'

> 1.4 ACRE-FT



22

AREA @ 532 90,000 D'

> 3.3 ACRE-FT



23

AREA @ 533 200,000 D'

> 4.6 ACRE-FT



24

AREA @ 534 200,000 D'

9.3 ACRE-FT



25

26

27

WATER ELEVATION = 6.6 ACRE-FT - 1.4 ACRE-FT - 3.3 ACRE-FT
4.6 ACRE-FT + 533

28

= 533.4 FT



29

30

31

32

33

34

35

* AREA SOUTH OF MAIN PLANT (FLYASH HANDLING AREA)



36

** ELEVATIONS FROM GOOGLE EARTH



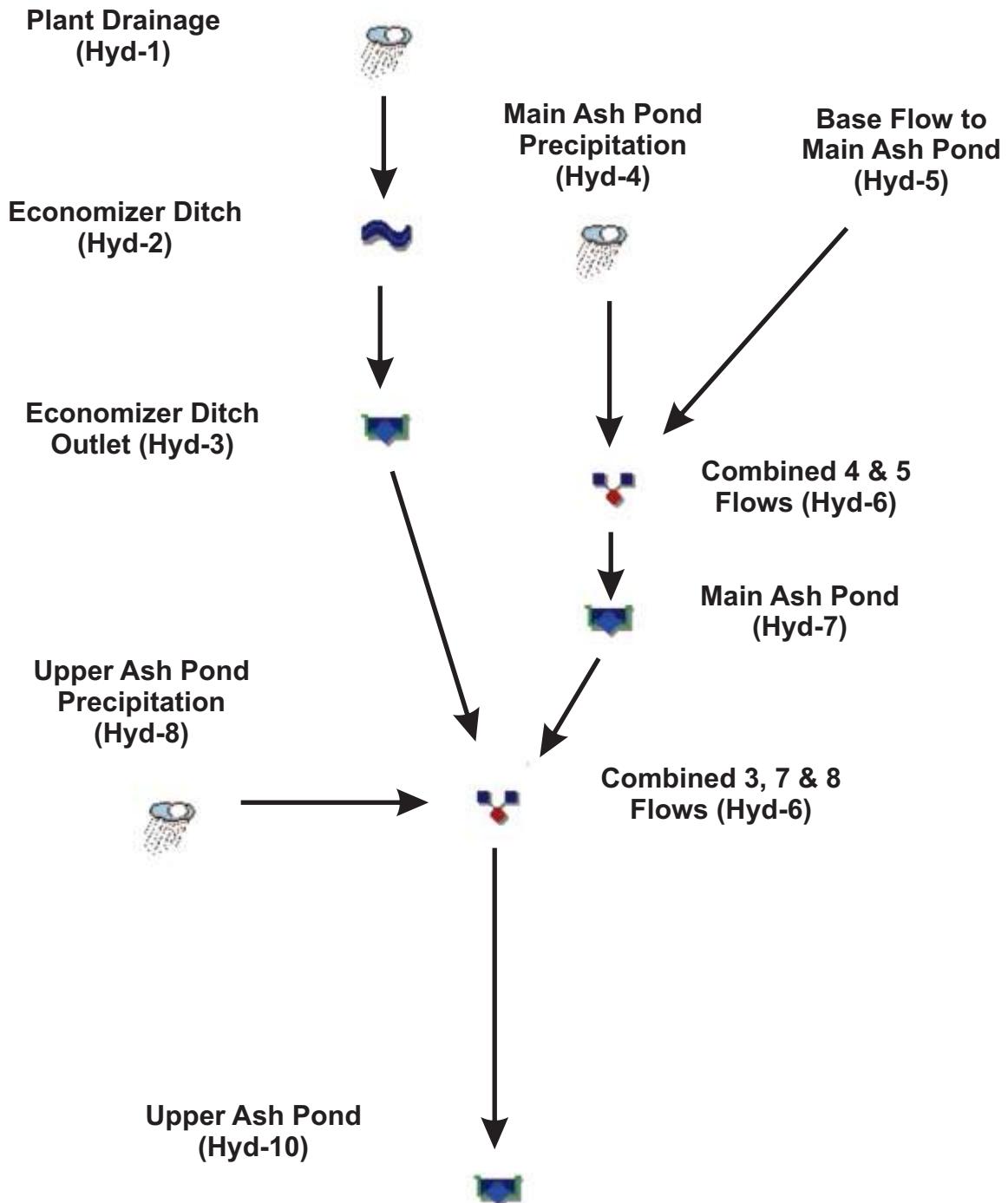
APPENDIX E – Ash Pond Hydraulic Analysis

Alliant Energy
Interstate Power and Light Company
Burlington Generating Station
Burlington, Iowa

Inflow Design Flood Control System Plan



HYDRAFLOW MODEL DIAGRAM



Hydrograph Summary Report

Page 1

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Maximum storage (acft)	Hydrograph description
1	Manual	19.30	4	704	8.743	----	-----	-----	Plant Area Drainage Pumps
2	Reach	19.30	4	836	8.780	1	-----	-----	Economizer Ditch
3	Reservoir	19.30	4	836	8.698	2	547.51	0.221	Econ. Ditch Outlet
4	SCS Runoff	136.99	4	728	12.342	----	-----	-----	Main Ash Pond Runoff
5	Manual	1.00	4	0	2.011	----	-----	-----	Main Base Flow
6	Combine	137.99	4	728	14.353	4, 5	-----	-----	Main Ash Pond Inflow
7	Reservoir	13.71	4	784	14.348	6	533.37	6.175	Main Ash Pond Outflow
8	SCS Runoff	237.81	4	724	18.127	----	-----	-----	Economizer Area Runoff
9	Combine	264.17	4	724	41.173	3, 7, 8	-----	-----	Upper Ash Pond Inflow
10	Reservoir	9.35	4	1460	41.023	9	530.23	29.621	Upper Ash Pond Outflow

Proj. file: Burlington-14.gpw

Return Period: 1,000 yr

Run date: 07-18-2017

Hydrograph Report

Page 1

Hydraflow Hydrographs by Intelisolve

Hyd. No. 1

Plant Area Drainage Pumps

Hydrograph type = Manual
Storm frequency = 1000 yrs

Peak discharge = 19.30 cfs
Time interval = 4 min

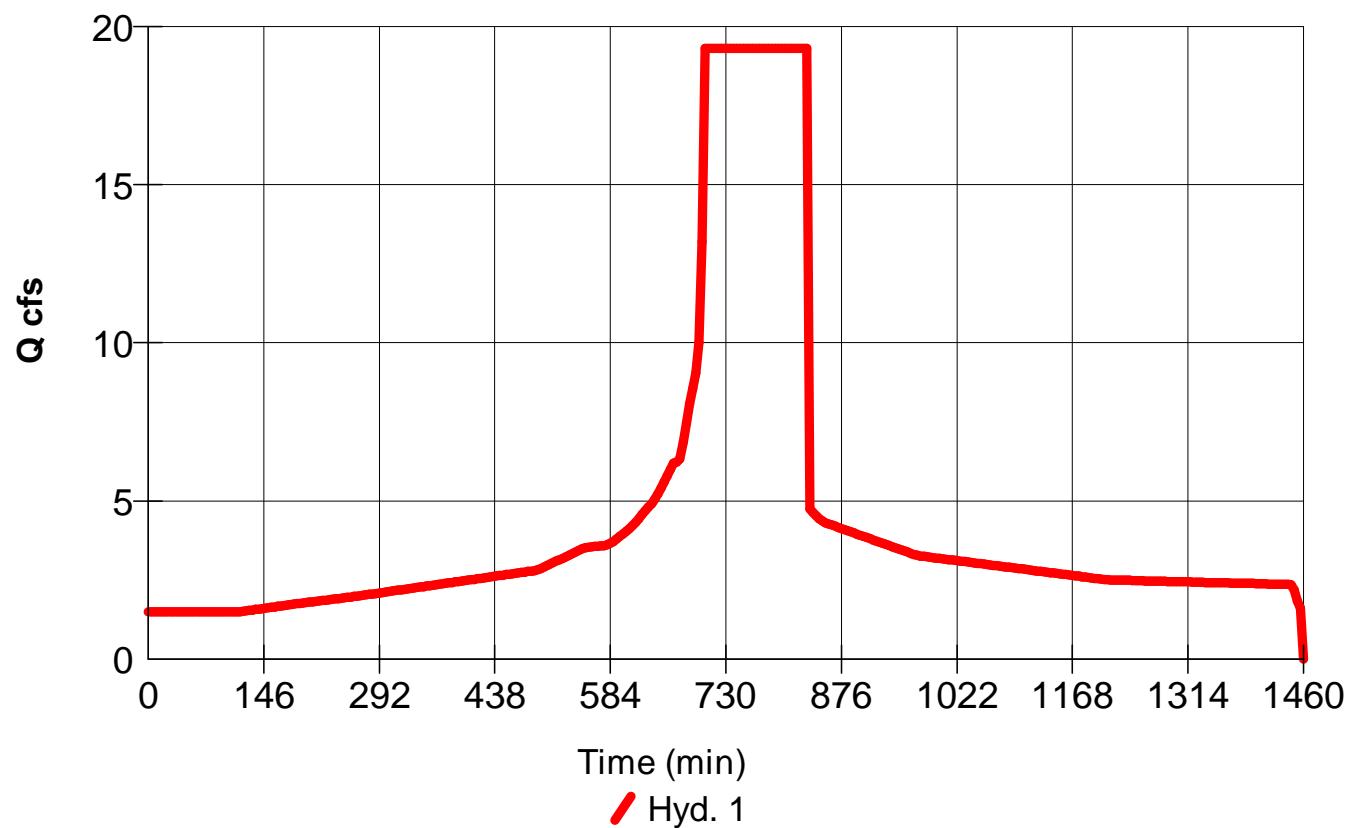
Hydrograph Volume = 8.743 acft

Hydrograph Discharge Table

Time -- Outflow (min	cfs)	Time -- Outflow (min	cfs)
-------------------------	------	-------------------------	------

248	1.94	928	3.66
268	2.00	948	3.48
288	2.07	968	3.31
308	2.15	988	3.22
328	2.22	1008	3.16
348	2.29	1028	3.09
368	2.36	1048	3.03
388	2.44	1068	2.96
408	2.51	1088	2.90
428	2.58	1108	2.84
448	2.65	1128	2.77
468	2.72	1148	2.71
488	2.80	1168	2.64
508	3.00	1188	2.58
528	3.24	1208	2.52
548	3.49	1228	2.49
568	3.57	1248	2.48
588	3.72	1268	2.47
608	4.12	1288	2.45
628	4.68	1308	2.44
648	5.41	1328	2.43
668	6.22	1348	2.42
688	8.55	1368	2.40
708	19.30 <<	1388	2.39
728	19.30 <<	1408	2.38
748	19.30 <<	1428	2.36
768	19.30 <<	1448	2.18
788	19.30 <<		
808	19.30 <<		
828	19.30 <<	...End	
848	4.44		
868	4.20		
888	4.02		
908	3.84		

Hyd. No. 1 - Manual - 1,000 Yr - $Q_p = 19.30 \text{ cfs}$ - Plant Area Drainage Pumps



Hydrograph Report

Page 1

Hydraflow Hydrographs by Intelisolve

Hyd. No. 2

Economizer Ditch

Hydrograph type	= Reach	Peak discharge	= 19.30 cfs
Storm frequency	= 1000 yrs	Time interval	= 4 min
Inflow hyd. No.	= 1	Section type	= Trapezoidal
Reach length	= 1000.0 ft	Channel slope	= 0.00 %
Manning's n	= 0.009	Bottom width	= 5.00 ft
Side slope	= 2.5:1	Max. depth	= 4.00 ft
Rating curve x	= 0.253	Rating curve m	= 1.350
Ave. velocity	= 0.78 ft/s	Routing coeff.	= 0.2240

Modified Att-Kin routing method used.

Hydrograph Volume = 8.780 acft

Hydrograph Discharge Table

Time (min)	Inflow cfs	Outflow cfs
280	2.05	1.98
300	2.12	2.05
320	2.19	2.13
340	2.26	2.20
360	2.34	2.27
380	2.41	2.34
400	2.48	2.42
420	2.55	2.49
440	2.62	2.56
460	2.70	2.63
480	2.77	2.70
500	2.91	2.78
520	3.14	2.96
540	3.39	3.18
560	3.55	3.41
580	3.61	3.52
600	3.96	3.70
620	4.44	4.04
640	5.06	4.54
660	6.00	5.24
680	7.46	6.08
700	13.20	8.13
720	19.30 <<	15.66
740	19.30 <<	18.28
760	19.30 <<	19.01
780	19.30 <<	19.22
800	19.30 <<	19.28
820	19.30 <<	19.29
840	4.64	16.04
860	4.27	7.69
880	4.09	5.17
900	3.91	4.33
920	3.73	3.97
940	3.55	3.74

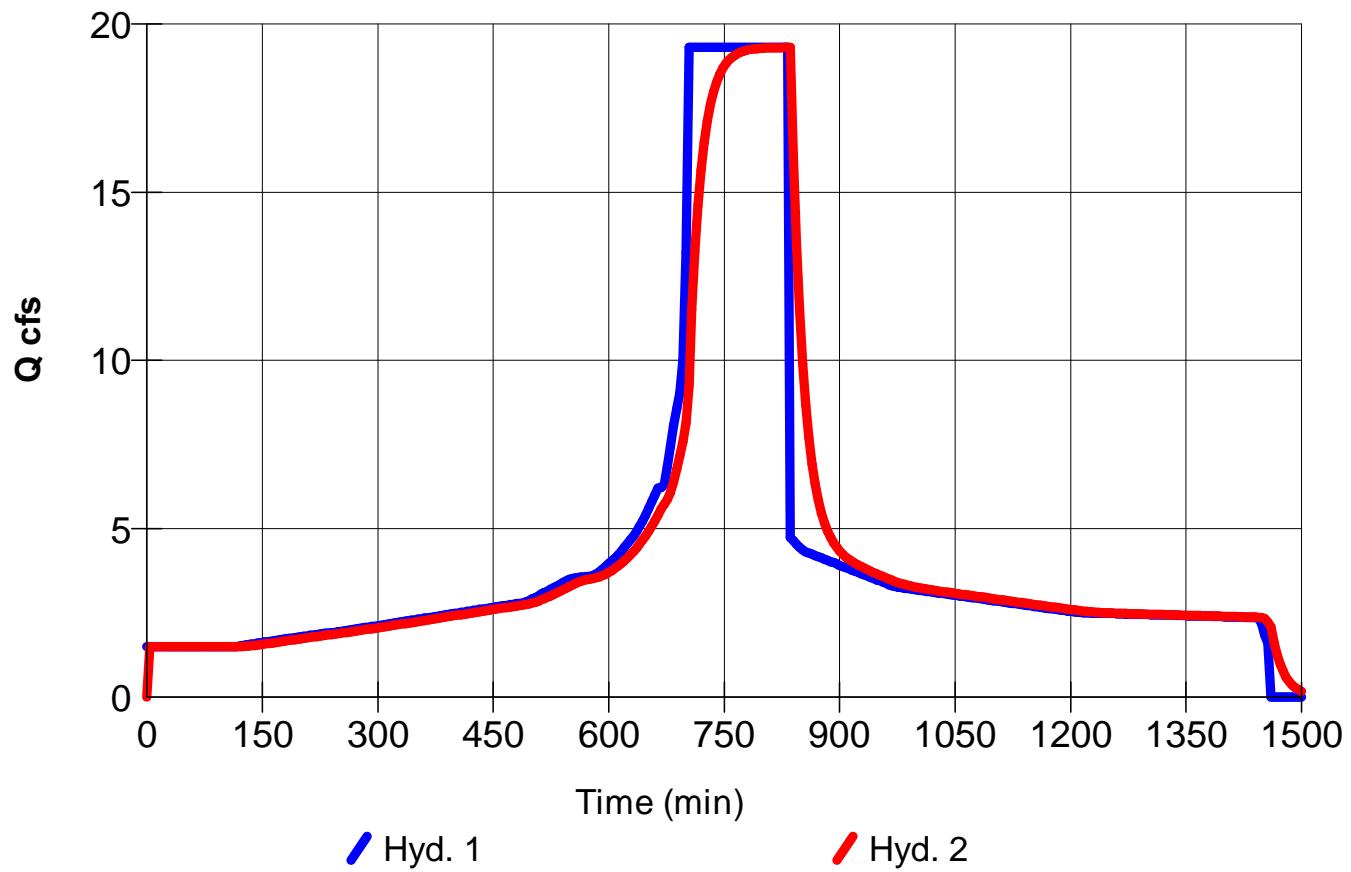
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Hydrograph Discharge Table

Time (min)	Inflow cfs	Outflow cfs
960	3.38	3.54
980	3.25	3.37
1000	3.18	3.26
1020	3.12	3.18
1040	3.05	3.11
1060	2.99	3.05
1080	2.93	2.98
1100	2.86	2.92
1120	2.80	2.85
1140	2.73	2.79
1160	2.67	2.73
1180	2.60	2.66
1200	2.54	2.60
1220	2.50	2.54
1240	2.49	2.51
1260	2.47	2.49
1280	2.46	2.47
1300	2.45	2.46
1320	2.43	2.45
1340	2.42	2.43
1360	2.41	2.42
1380	2.40	2.41
1400	2.38	2.40
1420	2.37	2.38
1440	2.36	2.37
1460	0.00	2.08

...End

Hyd. No. 2 - Reach - 1000 Yr - $Q_p = 19.30$ cfs - Economizer Ditch



Hydrograph Report

Page 1

Hydraflow Hydrographs by Intelisolve

Hyd. No. 3

Econ. Ditch Outlet

Hydrograph type	= Reservoir	Peak discharge	= 19.30 cfs
Storm frequency	= 1000 yrs	Time interval	= 4 min
Inflow hyd. No.	= 2	Reservoir name	= Economizer Outlet
Max. Elevation	= 547.51 ft	Max. Storage	= 0.221 acft

Storage Indication method used.

Outflow hydrograph volume = 8.698 acft

Hydrograph Discharge Table

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
280	1.98	544.75	----	----	----	----	----	----	----	----	----	1.97
300	2.05	544.76	----	----	----	----	----	----	----	----	----	2.04
320	2.13	544.77	----	----	----	----	----	----	----	----	----	2.11
340	2.20	544.78	----	----	----	----	----	----	----	----	----	2.18
360	2.27	544.79	----	----	----	----	----	----	----	----	----	2.26
380	2.34	544.80	----	----	----	----	----	----	----	----	----	2.33
400	2.42	544.81	----	----	----	----	----	----	----	----	----	2.40
420	2.49	544.82	----	----	----	----	----	----	----	----	----	2.47
440	2.56	544.83	----	----	----	----	----	----	----	----	----	2.55
460	2.63	544.84	----	----	----	----	----	----	----	----	----	2.62
480	2.70	544.85	----	----	----	----	----	----	----	----	----	2.69
500	2.78	544.86	----	----	----	----	----	----	----	----	----	2.77
520	2.96	544.88	----	----	----	----	----	----	----	----	----	2.92
540	3.18	544.91	----	----	----	----	----	----	----	----	----	3.14
560	3.41	544.94	----	----	----	----	----	----	----	----	----	3.37
580	3.52	544.96	----	----	----	----	----	----	----	----	----	3.51
600	3.70	544.98	----	----	----	----	----	----	----	----	----	3.66
620	4.04	545.02	----	----	----	----	----	----	----	----	----	3.94
640	4.54	545.11	----	----	----	----	----	----	----	----	----	4.39
660	5.24	545.24	----	----	----	----	----	----	----	----	----	5.01
680	6.08	545.43	----	----	----	----	----	----	----	----	----	5.73
700	8.13	545.83	----	----	----	----	----	----	----	----	----	7.10
720	15.66	547.01	----	----	----	----	----	----	----	----	----	14.24
740	18.28	547.37	----	----	----	----	----	----	----	----	----	17.87
760	19.01	547.47	----	----	----	----	----	----	----	----	----	18.89
780	19.22	547.50	----	----	----	----	----	----	----	----	----	19.19
800	19.28	547.51	----	----	----	----	----	----	----	----	----	19.27
820	19.29	547.51	----	----	----	----	----	----	----	----	----	19.29
840	16.04	547.41	----	----	----	----	----	----	----	----	----	18.31
860	7.69	546.49	----	----	----	----	----	----	----	----	----	8.86
880	5.17	545.62	----	----	----	----	----	----	----	----	----	6.41
900	4.33	545.17	----	----	----	----	----	----	----	----	----	4.63
920	3.97	545.05	----	----	----	----	----	----	----	----	----	4.07
940	3.74	544.99	----	----	----	----	----	----	----	----	----	3.79
960	3.54	544.97	----	----	----	----	----	----	----	----	----	3.57
980	3.37	544.94	----	----	----	----	----	----	----	----	----	3.40
1000	3.26	544.93	----	----	----	----	----	----	----	----	----	3.27
1020	3.18	544.91	----	----	----	----	----	----	----	----	----	3.19

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Hydrograph Discharge Table

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
1040	3.11	544.90	----	----	----	----	----	----	----	----	----	3.12
1060	3.05	544.90	----	----	----	----	----	----	----	----	----	3.06
1080	2.98	544.89	----	----	----	----	----	----	----	----	----	2.99
1100	2.92	544.88	----	----	----	----	----	----	----	----	----	2.93
1120	2.85	544.87	----	----	----	----	----	----	----	----	----	2.87
1140	2.79	544.86	----	----	----	----	----	----	----	----	----	2.80
1160	2.73	544.85	----	----	----	----	----	----	----	----	----	2.74
1180	2.66	544.84	----	----	----	----	----	----	----	----	----	2.67
1200	2.60	544.84	----	----	----	----	----	----	----	----	----	2.61
1220	2.54	544.83	----	----	----	----	----	----	----	----	----	2.55
1240	2.51	544.82	----	----	----	----	----	----	----	----	----	2.51
1260	2.49	544.82	----	----	----	----	----	----	----	----	----	2.49
1280	2.47	544.82	----	----	----	----	----	----	----	----	----	2.47
1300	2.46	544.81	----	----	----	----	----	----	----	----	----	2.46
1320	2.45	544.81	----	----	----	----	----	----	----	----	----	2.45
1340	2.43	544.81	----	----	----	----	----	----	----	----	----	2.43
1360	2.42	544.81	----	----	----	----	----	----	----	----	----	2.42
1380	2.41	544.81	----	----	----	----	----	----	----	----	----	2.41
1400	2.40	544.81	----	----	----	----	----	----	----	----	----	2.40
1420	2.38	544.80	----	----	----	----	----	----	----	----	----	2.38
1440	2.37	544.80	----	----	----	----	----	----	----	----	----	2.37
1460	2.08	544.78	----	----	----	----	----	----	----	----	----	2.19

...End

Reservoir Report

Page 1

Reservoir No. 1 - Economizer Outlet

Hydraflow Hydrographs by Intelisolve

Pond Data

Pond storage is based on known values

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (acft)	Total storage (acft)
0.00	541.00	00	0.000	0.000
0.75	541.75	00	0.009	0.009
1.50	542.50	00	0.012	0.021
2.25	543.25	00	0.016	0.037
3.00	544.00	00	0.020	0.057
3.50	544.50	00	0.016	0.073
3.75	544.75	00	0.009	0.082
4.00	545.00	00	0.009	0.091
4.25	545.25	00	0.010	0.101
4.50	545.50	00	0.011	0.112
4.75	545.75	00	0.011	0.123
5.00	546.00	00	0.012	0.135
5.25	546.25	00	0.012	0.147
5.50	546.50	00	0.013	0.160
5.75	546.75	00	0.014	0.174
6.00	547.00	00	0.015	0.189
6.25	547.25	00	0.015	0.204
6.50	547.50	00	0.016	0.220
6.75	547.75	00	0.017	0.237
7.00	548.00	00	0.018	0.255

Stage / Storage / Discharge Table

Note: All outflows have been analyzed under inlet and outlet control.

Stage ft	Storage acft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Total cfs
0.00	0.000	541.00	0.00	0.00	---	---	0.00	---	---	---	---	0.00
0.08	0.001	541.08	0.00	0.00	---	---	0.00	---	---	---	---	0.00
0.15	0.002	541.15	0.00	0.00	---	---	0.00	---	---	---	---	0.00
0.23	0.003	541.23	0.00	0.00	---	---	0.00	---	---	---	---	0.00
0.30	0.004	541.30	0.00	0.00	---	---	0.00	---	---	---	---	0.00
0.38	0.005	541.38	0.00	0.00	---	---	0.00	---	---	---	---	0.00
0.45	0.005	541.45	0.00	0.00	---	---	0.00	---	---	---	---	0.00
0.53	0.006	541.53	0.00	0.00	---	---	0.00	---	---	---	---	0.00
0.60	0.007	541.60	0.00	0.00	---	---	0.00	---	---	---	---	0.00
0.68	0.008	541.68	0.00	0.00	---	---	0.00	---	---	---	---	0.00
0.75	0.009	541.75	0.00	0.00	---	---	0.00	---	---	---	---	0.00
0.83	0.010	541.83	0.00	0.00	---	---	0.00	---	---	---	---	0.00
0.90	0.011	541.90	0.00	0.00	---	---	0.00	---	---	---	---	0.00
0.98	0.013	541.98	0.00	0.00	---	---	0.00	---	---	---	---	0.00
1.05	0.014	542.05	0.00	0.00	---	---	0.00	---	---	---	---	0.00
1.13	0.015	542.13	0.00	0.00	---	---	0.00	---	---	---	---	0.00
1.20	0.016	542.20	0.00	0.00	---	---	0.00	---	---	---	---	0.00
1.28	0.017	542.28	0.00	0.00	---	---	0.00	---	---	---	---	0.00

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Economizer Outlet
Stage / Storage / Discharge Table

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Stage ft	Storage acft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Total cfs
1.35	0.019	542.35	0.00	0.00	---	---	0.00	---	---	---	---	0.00
1.43	0.020	542.43	0.00	0.00	---	---	0.00	---	---	---	---	0.00
1.50	0.021	542.50	0.00	0.00	---	---	0.00	---	---	---	---	0.00
1.58	0.023	542.58	0.00	0.00	---	---	0.00	---	---	---	---	0.00
1.65	0.024	542.65	0.00	0.00	---	---	0.00	---	---	---	---	0.00
1.73	0.026	542.73	0.00	0.00	---	---	0.00	---	---	---	---	0.00
1.80	0.027	542.80	0.00	0.00	---	---	0.00	---	---	---	---	0.00
1.88	0.029	542.88	0.00	0.00	---	---	0.00	---	---	---	---	0.00
1.95	0.031	542.95	0.00	0.00	---	---	0.00	---	---	---	---	0.00
2.03	0.032	543.03	0.00	0.00	---	---	0.00	---	---	---	---	0.00
2.10	0.034	543.10	0.00	0.00	---	---	0.00	---	---	---	---	0.00
2.18	0.035	543.18	0.00	0.00	---	---	0.00	---	---	---	---	0.00
2.25	0.037	543.25	0.00	0.00	---	---	0.00	---	---	---	---	0.00
2.33	0.039	543.33	0.00	0.00	---	---	0.00	---	---	---	---	0.00
2.40	0.041	543.40	0.00	0.00	---	---	0.00	---	---	---	---	0.00
2.48	0.043	543.48	0.00	0.00	---	---	0.00	---	---	---	---	0.00
2.55	0.045	543.55	0.00	0.00	---	---	0.00	---	---	---	---	0.00
2.63	0.047	543.63	0.00	0.00	---	---	0.00	---	---	---	---	0.00
2.70	0.049	543.70	0.00	0.00	---	---	0.00	---	---	---	---	0.00
2.78	0.051	543.78	0.00	0.00	---	---	0.00	---	---	---	---	0.00
2.85	0.053	543.85	0.00	0.00	---	---	0.00	---	---	---	---	0.00
2.93	0.055	543.93	0.00	0.00	---	---	0.00	---	---	---	---	0.00
3.00	0.057	544.00	0.00	0.00	---	---	0.00	---	---	---	---	0.00
3.05	0.059	544.05	0.00	0.00	---	---	0.00	---	---	---	---	0.00
3.10	0.060	544.10	0.00	0.00	---	---	0.00	---	---	---	---	0.00
3.15	0.062	544.15	0.00	0.00	---	---	0.00	---	---	---	---	0.00
3.20	0.063	544.20	0.00	0.00	---	---	0.00	---	---	---	---	0.00
3.25	0.065	544.25	0.00	0.00	---	---	0.00	---	---	---	---	0.00
3.30	0.067	544.30	0.00	0.00	---	---	0.00	---	---	---	---	0.00
3.35	0.068	544.35	0.00	0.00	---	---	0.00	---	---	---	---	0.00
3.40	0.070	544.40	0.00	0.00	---	---	0.00	---	---	---	---	0.00
3.45	0.071	544.45	0.00	0.00	---	---	0.00	---	---	---	---	0.00
3.50	0.073	544.50	0.00	0.00	---	---	0.00	---	---	---	---	0.00
3.53	0.074	544.53	0.00	0.00	---	---	0.00	---	---	---	---	0.20
3.55	0.075	544.55	0.00	0.00	---	---	0.00	---	---	---	---	0.40
3.58	0.076	544.58	0.00	0.00	---	---	0.00	---	---	---	---	0.59
3.60	0.077	544.60	0.00	0.00	---	---	0.00	---	---	---	---	0.79
3.63	0.077	544.63	0.00	0.00	---	---	0.00	---	---	---	---	0.99
3.65	0.078	544.65	0.00	0.00	---	---	0.00	---	---	---	---	1.19
3.68	0.079	544.68	0.00	0.00	---	---	0.00	---	---	---	---	1.39
3.70	0.080	544.70	0.00	0.00	---	---	0.00	---	---	---	---	1.58
3.73	0.081	544.73	0.00	0.00	---	---	0.00	---	---	---	---	1.78
3.75	0.082	544.75	0.00	0.00	---	---	0.00	---	---	---	---	1.98
3.78	0.083	544.78	0.00	0.00	---	---	0.00	---	---	---	---	2.17
3.80	0.084	544.80	0.00	0.00	---	---	0.00	---	---	---	---	2.35
3.83	0.085	544.83	0.00	0.00	---	---	0.00	---	---	---	---	2.54
3.85	0.086	544.85	0.00	0.00	---	---	0.00	---	---	---	---	2.72
3.88	0.086	544.88	0.00	0.00	---	---	0.00	---	---	---	---	2.91
3.90	0.087	544.90	0.00	0.00	---	---	0.00	---	---	---	---	3.09
3.93	0.088	544.93	0.00	0.00	---	---	0.00	---	---	---	---	3.28
3.95	0.089	544.95	0.00	0.00	---	---	0.00	---	---	---	---	3.46
3.98	0.090	544.98	0.00	0.00	---	---	0.00	---	---	---	---	3.65
4.00	0.091	545.00	0.00	0.00	---	---	0.00	---	---	---	---	3.83
4.03	0.092	545.03	0.00	0.00	---	---	0.00	---	---	---	---	3.95
4.05	0.093	545.05	0.00	0.00	---	---	0.00	---	---	---	---	4.07
4.08	0.094	545.08	0.00	0.00	---	---	0.00	---	---	---	---	4.19
4.10	0.095	545.10	0.00	0.00	---	---	0.00	---	---	---	---	4.31
4.13	0.096	545.13	0.00	0.00	---	---	0.00	---	---	---	---	4.43
4.15	0.097	545.15	0.00	0.00	---	---	0.00	---	---	---	---	4.56
4.18	0.098	545.18	0.00	0.00	---	---	0.00	---	---	---	---	4.68
4.20	0.099	545.20	0.00	0.00	---	---	0.00	---	---	---	---	4.80
4.23	0.100	545.23	0.00	0.00	---	---	0.00	---	---	---	---	4.92
4.25	0.101	545.25	0.00	0.00	---	---	0.00	---	---	---	---	5.04
4.28	0.102	545.28	0.00	0.00	---	---	0.00	---	---	---	---	5.14
4.30	0.103	545.30	0.00	0.00	---	---	0.00	---	---	---	---	5.23
4.33	0.104	545.33	0.00	0.00	---	---	0.00	---	---	---	---	5.33
4.35	0.105	545.35	0.00	0.00	---	---	0.00	---	---	---	---	5.43
4.38	0.107	545.38	0.00	0.00	---	---	0.00	---	---	---	---	5.53
4.40	0.108	545.40	0.00	0.00	---	---	0.00	---	---	---	---	5.62
4.43	0.109	545.43	0.00	0.00	---	---	0.00	---	---	---	---	5.72

Continues on next page...

Economizer Outlet
Stage / Storage / Discharge Table

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Stage ft	Storage acft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Total cfs
4.45	0.110	545.45	0.00	0.00	---	---	0.00	---	---	---	---	5.82
4.48	0.111	545.48	0.00	0.00	---	---	0.00	---	---	---	---	5.91
4.50	0.112	545.50	0.00	0.00	---	---	0.00	---	---	---	---	6.01
4.53	0.113	545.53	0.00	0.00	---	---	0.00	---	---	---	---	6.09
4.55	0.114	545.55	0.00	0.00	---	---	0.00	---	---	---	---	6.18
4.58	0.115	545.58	0.00	0.00	---	---	0.00	---	---	---	---	6.26
4.60	0.116	545.60	0.00	0.00	---	---	0.00	---	---	---	---	6.35
4.63	0.118	545.63	0.00	0.00	---	---	0.00	---	---	---	---	6.43
4.65	0.119	545.65	0.00	0.00	---	---	0.00	---	---	---	---	6.51
4.68	0.120	545.68	0.00	0.00	---	---	0.00	---	---	---	---	6.60
4.70	0.121	545.70	0.00	0.00	---	---	0.00	---	---	---	---	6.68
4.73	0.122	545.73	0.00	0.00	---	---	0.00	---	---	---	---	6.77
4.75	0.123	545.75	0.00	0.00	---	---	0.00	---	---	---	---	6.85
4.78	0.124	545.78	0.00	0.00	---	---	0.00	---	---	---	---	6.92
4.80	0.125	545.80	0.00	0.00	---	---	0.00	---	---	---	---	7.00
4.83	0.127	545.83	0.00	0.00	---	---	0.00	---	---	---	---	7.07
4.85	0.128	545.85	0.00	0.00	---	---	0.00	---	---	---	---	7.15
4.88	0.129	545.88	0.00	0.00	---	---	0.00	---	---	---	---	7.22
4.90	0.130	545.90	0.00	0.00	---	---	0.00	---	---	---	---	7.29
4.93	0.131	545.93	0.00	0.00	---	---	0.00	---	---	---	---	7.37
4.95	0.133	545.95	0.00	0.00	---	---	0.00	---	---	---	---	7.44
4.98	0.134	545.98	0.00	0.00	---	---	0.00	---	---	---	---	7.52
5.00	0.135	546.00	0.00	0.00	---	---	0.00	---	---	---	---	7.59
5.03	0.136	546.03	0.00	0.00	---	---	0.00	---	---	---	---	7.66
5.05	0.137	546.05	0.00	0.00	---	---	0.00	---	---	---	---	7.73
5.08	0.139	546.08	0.00	0.00	---	---	0.00	---	---	---	---	7.79
5.10	0.140	546.10	0.00	0.00	---	---	0.00	---	---	---	---	7.86
5.13	0.141	546.13	0.00	0.00	---	---	0.00	---	---	---	---	7.93
5.15	0.142	546.15	0.00	0.00	---	---	0.00	---	---	---	---	8.00
5.18	0.143	546.18	0.00	0.00	---	---	0.00	---	---	---	---	8.07
5.20	0.145	546.20	0.00	0.00	---	---	0.00	---	---	---	---	8.13
5.23	0.146	546.23	0.00	0.00	---	---	0.00	---	---	---	---	8.20
5.25	0.147	546.25	0.00	0.00	---	---	0.00	---	---	---	---	8.27
5.28	0.148	546.28	0.00	0.00	---	---	0.00	---	---	---	---	8.33
5.30	0.150	546.30	0.00	0.00	---	---	0.00	---	---	---	---	8.39
5.33	0.151	546.33	0.00	0.00	---	---	0.00	---	---	---	---	8.46
5.35	0.152	546.35	0.00	0.00	---	---	0.00	---	---	---	---	8.52
5.38	0.154	546.38	0.00	0.00	---	---	0.00	---	---	---	---	8.58
5.40	0.155	546.40	0.00	0.00	---	---	0.00	---	---	---	---	8.64
5.43	0.156	546.43	0.00	0.00	---	---	0.00	---	---	---	---	8.70
5.45	0.157	546.45	0.00	0.00	---	---	0.00	---	---	---	---	8.77
5.48	0.159	546.48	0.00	0.00	---	---	0.00	---	---	---	---	8.83
5.50	0.160	546.50	0.00	0.00	---	---	0.00	---	---	---	---	8.89
5.53	0.161	546.53	0.00	0.00	---	---	0.00	---	---	---	---	9.15
5.55	0.163	546.55	0.00	0.00	---	---	0.00	---	---	---	---	9.42
5.58	0.164	546.58	0.00	0.00	---	---	0.00	---	---	---	---	9.68
5.60	0.166	546.60	0.00	0.00	---	---	0.00	---	---	---	---	9.94
5.63	0.167	546.63	0.00	0.00	---	---	0.00	---	---	---	---	10.21
5.65	0.168	546.65	0.00	0.00	---	---	0.00	---	---	---	---	10.47
5.68	0.170	546.68	0.00	0.00	---	---	0.00	---	---	---	---	10.73
5.70	0.171	546.70	0.00	0.00	---	---	0.00	---	---	---	---	10.99
5.73	0.173	546.73	0.00	0.00	---	---	0.00	---	---	---	---	11.26
5.75	0.174	546.75	0.00	0.00	---	---	0.00	---	---	---	---	11.52
5.78	0.176	546.78	0.00	0.00	---	---	0.00	---	---	---	---	11.78
5.80	0.177	546.80	0.00	0.00	---	---	0.00	---	---	---	---	12.04
5.83	0.179	546.83	0.00	0.00	---	---	0.00	---	---	---	---	12.30
5.85	0.180	546.85	0.00	0.00	---	---	0.00	---	---	---	---	12.56
5.88	0.182	546.88	0.00	0.00	---	---	0.00	---	---	---	---	12.82
5.90	0.183	546.90	0.00	0.00	---	---	0.00	---	---	---	---	13.07
5.93	0.185	546.93	0.00	0.00	---	---	0.00	---	---	---	---	13.33
5.95	0.186	546.95	0.00	0.00	---	---	0.00	---	---	---	---	13.59
5.98	0.188	546.98	0.00	0.00	---	---	0.00	---	---	---	---	13.85
6.00	0.189	547.00	0.00	0.00	---	---	0.00	---	---	---	---	14.11
6.03	0.191	547.03	0.00	0.00	---	---	0.00	---	---	---	---	14.37
6.05	0.192	547.05	0.00	0.00	---	---	0.00	---	---	---	---	14.62
6.08	0.194	547.08	0.00	0.00	---	---	0.00	---	---	---	---	14.88
6.10	0.195	547.10	0.00	0.00	---	---	0.00	---	---	---	---	15.13
6.13	0.197	547.13	0.00	0.00	---	---	0.00	---	---	---	---	15.39
6.15	0.198	547.15	0.00	0.00	---	---	0.00	---	---	---	---	15.65
6.18	0.200	547.18	0.00	0.00	---	---	0.00	---	---	---	---	15.90

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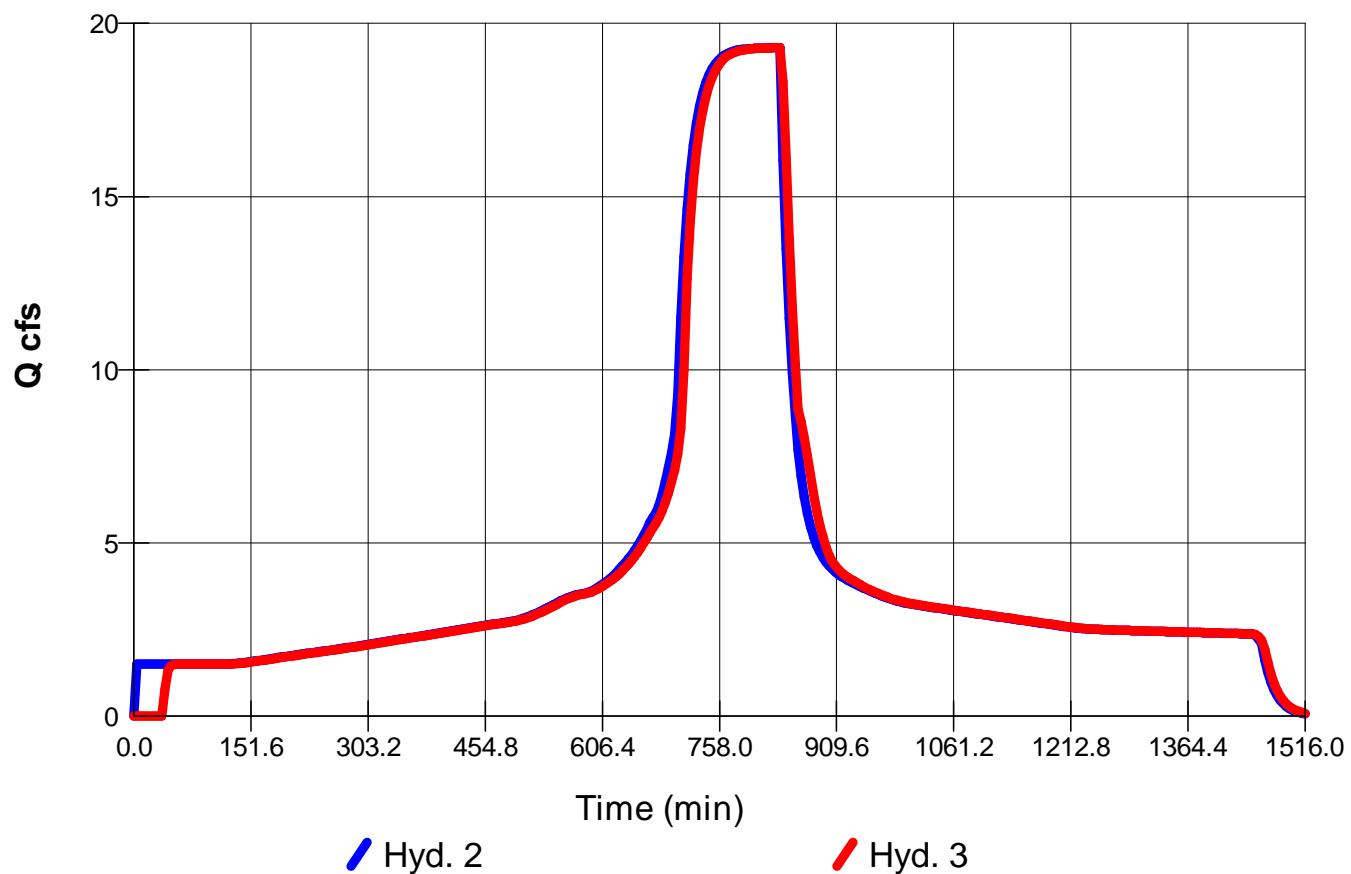
Economizer Outlet
Stage / Storage / Discharge Table

Page 4

Stage ft	Storage acft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Total cfs
6.20	0.201	547.20	0.00	0.00	---	---	0.00	---	---	---	---	16.16
6.23	0.203	547.23	0.00	0.00	---	---	0.00	---	---	---	---	16.41
6.25	0.204	547.25	0.00	0.00	---	---	0.00	---	---	---	---	16.67
6.28	0.206	547.28	0.00	0.00	---	---	0.00	---	---	---	---	16.92
6.30	0.207	547.30	0.00	0.00	---	---	0.00	---	---	---	---	17.18
6.33	0.209	547.33	0.00	0.00	---	---	0.00	---	---	---	---	17.43
6.35	0.210	547.35	0.00	0.00	---	---	0.00	---	---	---	---	17.68
6.38	0.212	547.38	0.00	0.00	---	---	0.00	---	---	---	---	17.94
6.40	0.214	547.40	0.00	0.00	---	---	0.00	---	---	---	---	18.19
6.43	0.215	547.43	0.00	0.00	---	---	0.00	---	---	---	---	18.44
6.45	0.217	547.45	0.00	0.00	---	---	0.00	---	---	---	---	18.69
6.48	0.218	547.48	0.00	0.00	---	---	0.00	---	---	---	---	18.95
6.50	0.220	547.50	0.00	0.00	---	---	0.00	---	---	---	---	19.20
6.53	0.222	547.53	0.00	0.00	---	---	0.00	---	---	---	---	19.45
6.55	0.223	547.55	0.00	0.00	---	---	0.00	---	---	---	---	19.70
6.58	0.225	547.58	0.00	0.00	---	---	0.00	---	---	---	---	19.96
6.60	0.227	547.60	0.00	0.00	---	---	0.00	---	---	---	---	20.21
6.63	0.229	547.63	0.00	0.00	---	---	0.00	---	---	---	---	20.46
6.65	0.230	547.65	0.00	0.00	---	---	0.00	---	---	---	---	20.71
6.68	0.232	547.68	0.00	0.00	---	---	0.00	---	---	---	---	20.96
6.70	0.234	547.70	0.00	0.00	---	---	0.00	---	---	---	---	21.22
6.73	0.235	547.73	0.00	0.00	---	---	0.00	---	---	---	---	21.47
6.75	0.237	547.75	0.00	0.00	---	---	0.00	---	---	---	---	21.72
6.78	0.239	547.78	0.00	0.00	---	---	0.00	---	---	---	---	21.97
6.80	0.241	547.80	0.00	0.00	---	---	0.00	---	---	---	---	22.22
6.83	0.242	547.83	0.00	0.00	---	---	0.00	---	---	---	---	22.47
6.85	0.244	547.85	0.00	0.00	---	---	0.00	---	---	---	---	22.72
6.88	0.246	547.88	0.00	0.00	---	---	0.00	---	---	---	---	22.97
6.90	0.248	547.90	0.00	0.00	---	---	0.00	---	---	---	---	23.21
6.93	0.250	547.93	0.00	0.00	---	---	0.00	---	---	---	---	23.46
6.95	0.251	547.95	0.00	0.00	---	---	0.00	---	---	---	---	23.71
6.98	0.253	547.98	0.00	0.00	---	---	0.00	---	---	---	---	23.96
7.00	0.255	548.00	0.00	0.00	---	---	0.00	---	---	---	---	24.21

...End

Hyd. No. 3 - Reservoir - 1000 Yr - $Q_p = 19.30 \text{ cfs}$ - Econ. Ditch Outlet



Burlington Station As Built - Economizer Ditch Outlet

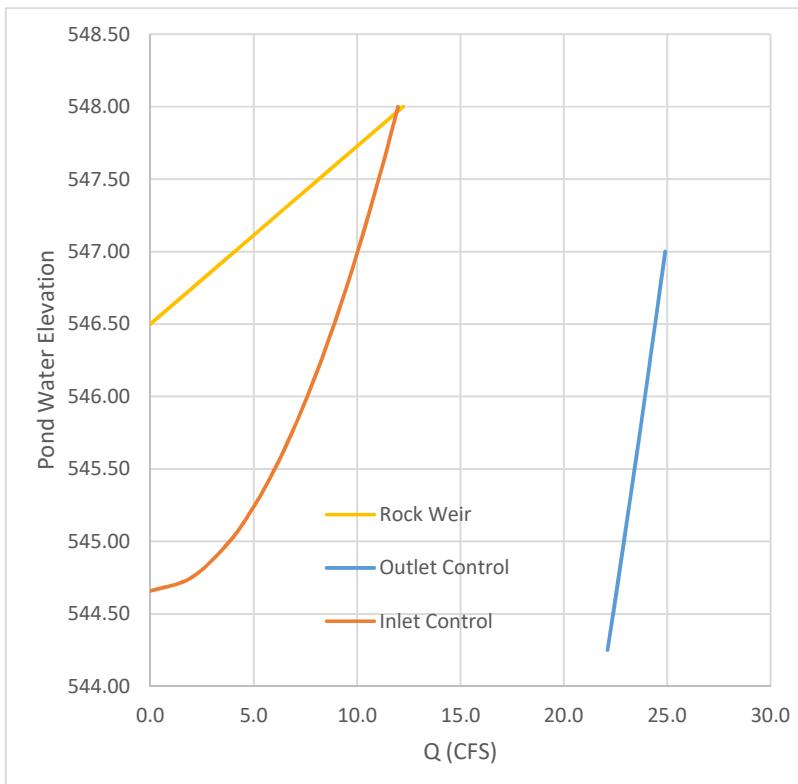
Hydrograph 3 Input

SUMMERGED OUTLET

1.3167 FT = Pipe Diameter Steel
 0.3292 FT = Hydraulic Radius
 2.4998 k in Q outlet control equation
 544 FT = Pipe Inlet Invert Elevation
 65 FT = Length
 0.011 = n for PVC
 0.6 = Co for Inlet Control
 534 FT = Tail Water Elevation
 12 FT = Weir Width
 0.5 Ft = d50
 0.45 Porosity

OUTLET CONTROL / BARROW CONTROL

Q	Elevation	Head
CFS		Ft
22.12	544.25	10.3
22.38	544.50	10.5
22.65	544.75	10.8
22.91	545.00	11.0
23.17	545.25	11.3
23.43	545.50	11.5
23.68	545.75	11.8
23.93	546.00	12.0
24.18	546.25	12.3
24.42	546.50	12.5
24.67	546.75	12.8
24.91	547.00	13.0



Velocity Equation

$$V = 2.48 * \text{SQRT}(G * d50) * (S^{0.58} / Cu^{2.22})$$

Assume Cu = 2

$$1.51 \text{ FPS} = V$$

Rock Filled Weir Flow

COMBINED FLOW

INLET CONTROL

Q	Elevation	Head (to pipe middle)
CFS		Ft
0.00	544.66	0.00
1.98	544.75	0.09
3.83	545.00	0.34
5.04	545.25	0.59
6.01	545.50	0.84
6.85	545.75	1.09
7.59	546.00	1.34
8.27	546.25	1.59
8.89	546.50	1.84
9.48	546.75	2.09
10.03	547.00	2.34
10.55	547.25	2.59
11.05	547.50	2.84
11.52	547.75	3.09
11.98	548.00	3.34

Ignored side slope rip-rap areas

H	Q	
Ft	CFS	
546.50	0.00	Inlet Only
544.66	0.00	
544.75	1.98	
545.00	3.83	
545.25	5.04	
545.50	6.01	
545.75	6.85	
546.00	7.59	
546.25	8.27	
546.50	8.89	Combined
546.75	11.52	
547.00	14.11	
547.25	16.67	
547.50	19.20	
547.75	21.72	
548.00	24.21	

Hydrograph Report

Page 1

Hydraflow Hydrographs by Intelisolve

Hyd. No. 4

Main Ash Pond Runoff

Hydrograph type	= SCS Runoff	Peak discharge	= 136.99 cfs
Storm frequency	= 100 yrs	Time interval	= 4 min
Drainage area	= 17.00 ac	Curve number	= 85
Basin Slope	= 1.5 %	Hydraulic length	= 1250 ft
Tc method	= LAG	Time of conc. (Tc)	= 26.3 min
Total precip.	= 10.30 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

Hydrograph Volume = 12.342 acft

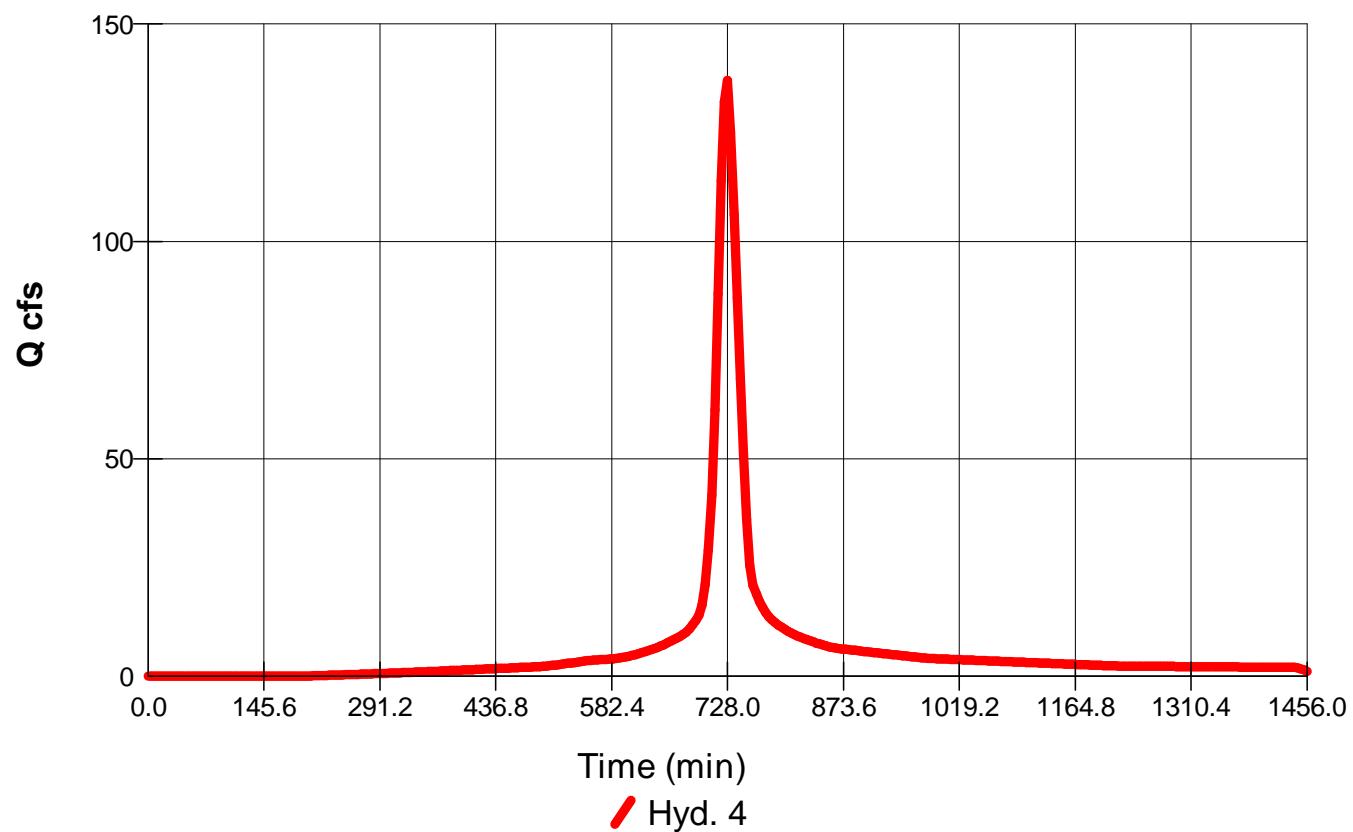
Hydrograph Discharge Table

Time -- Outflow
(min cfs)

692	14.02
712	61.16
732	124.99
752	35.97
772	15.73

...End

Hyd. No. 4 - SCS Runoff - 1000 Yr - $Q_p = 136.99 \text{ cfs}$ - Main Ash Pond
Runoff



Hydrograph Report

Page 1

Hydraflow Hydrographs by Intelisolve

Hyd. No. 5

Main Base Flow

Hydrograph type = Manual
Storm frequency = 1000 yrs

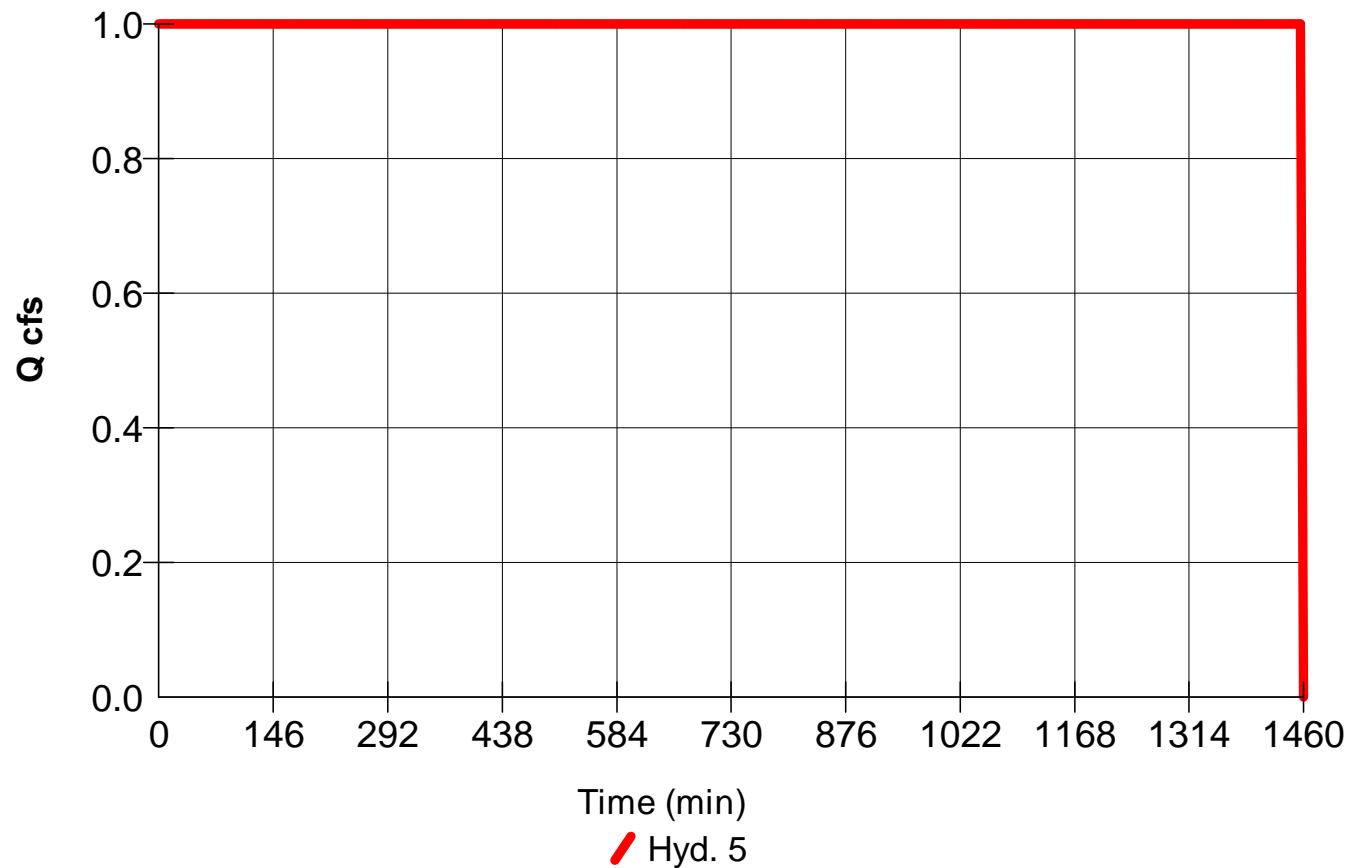
Peak discharge = 1.00 cfs
Time interval = 4 min

Hydrograph Volume = 2.011 acft

Hydrograph Discharge Table

Time -- Outflow (min	Outflow cfs)	Time -- Outflow (min	Outflow cfs)	Time -- Outflow (min	Outflow cfs)
0	1.00 <<	680	1.00 <<	1360	1.00 <<
20	1.00 <<	700	1.00 <<	1380	1.00 <<
40	1.00 <<	720	1.00 <<	1400	1.00 <<
60	1.00 <<	740	1.00 <<	1420	1.00 <<
80	1.00 <<	760	1.00 <<	1440	1.00 <<
100	1.00 <<	780	1.00 <<		
120	1.00 <<	800	1.00 <<		
140	1.00 <<	820	1.00 <<	...End	
160	1.00 <<	840	1.00 <<		
180	1.00 <<	860	1.00 <<		
200	1.00 <<	880	1.00 <<		
220	1.00 <<	900	1.00 <<		
240	1.00 <<	920	1.00 <<		
260	1.00 <<	940	1.00 <<		
280	1.00 <<	960	1.00 <<		
300	1.00 <<	980	1.00 <<		
320	1.00 <<	1000	1.00 <<		
340	1.00 <<	1020	1.00 <<		
360	1.00 <<	1040	1.00 <<		
380	1.00 <<	1060	1.00 <<		
400	1.00 <<	1080	1.00 <<		
420	1.00 <<	1100	1.00 <<		
440	1.00 <<	1120	1.00 <<		
460	1.00 <<	1140	1.00 <<		
480	1.00 <<	1160	1.00 <<		
500	1.00 <<	1180	1.00 <<		
520	1.00 <<	1200	1.00 <<		
540	1.00 <<	1220	1.00 <<		
560	1.00 <<	1240	1.00 <<		
580	1.00 <<	1260	1.00 <<		
600	1.00 <<	1280	1.00 <<		
620	1.00 <<	1300	1.00 <<		
640	1.00 <<	1320	1.00 <<		
660	1.00 <<	1340	1.00 <<		

Hyd. No. 5 - Manual - 1000 Yr - $Q_p = 1.00 \text{ cfs}$ - Main Base Flow



Hydrograph Report

Page 1

Hydraflow Hydrographs by Intelisolve

Hyd. No. 6

Main Ash Pond Inflow

Hydrograph type = Combine
Storm frequency = 1000 yrs
Inflow hyds. = 4, 5

Peak discharge = 137.99 cfs
Time interval = 4 min

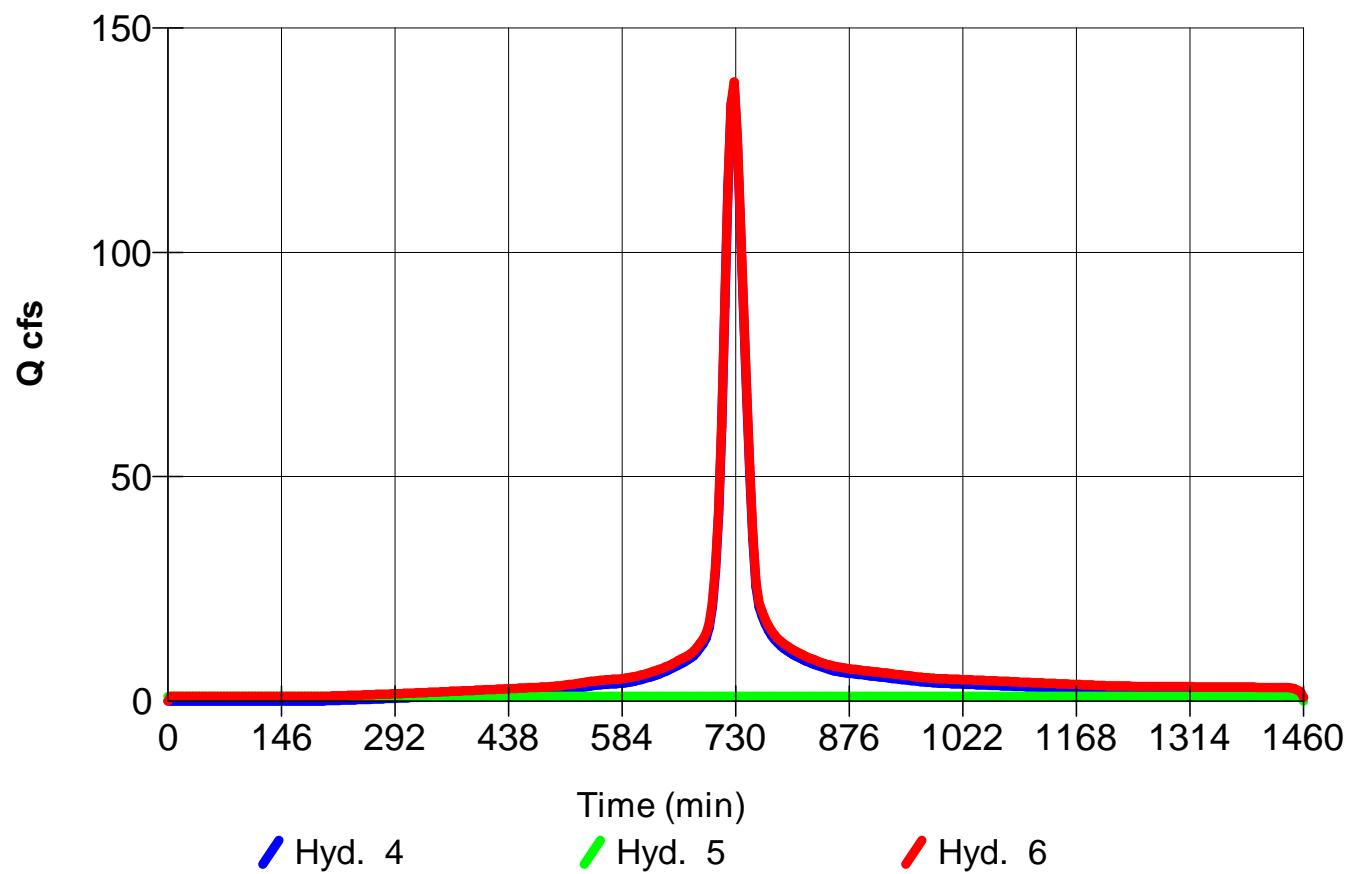
Hydrograph Volume = 14.353 acft

Hydrograph Discharge Table

Time (min)	Hyd. 4 + (cfs)	Hyd. 5 = (cfs)	Outflow (cfs)
700	20.99	1.00 <<	21.99
720	113.94	1.00 <<	114.94
740	86.98	1.00 <<	87.98
760	20.92	1.00 <<	21.92
780	13.66	1.00 <<	14.66

...End

Hyd. No. 6 - Combine - 1000 Yr - $Q_p = 137.99$ cfs - Main Ash Pond Inflow



Hydrograph Report

Page 1

Hydraflow Hydrographs by Intelisolve

Hyd. No. 7

Main Ash Pond Outflow

Hydrograph type	= Reservoir	Peak discharge	= 13.71 cfs
Storm frequency	= 1000 yrs	Time interval	= 4 min
Inflow hyd. No.	= 6	Reservoir name	= Main Ash Pond
Max. Elevation	= 533.37 ft	Max. Storage	= 6.175 acft

Storage Indication method used.

Outflow hydrograph volume = 14.348 acft

Hydrograph Discharge Table

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
300	1.65	531.42	1.39	----	----	----	----	----	----	----	----	1.39
320	1.79	531.44	1.52	----	----	----	----	----	----	----	----	1.52
340	1.94	531.46	1.65	----	----	----	----	----	----	----	----	1.65
360	2.09	531.47	1.80	----	----	----	----	----	----	----	----	1.80
380	2.25	531.49	1.96	----	----	----	----	----	----	----	----	1.96
400	2.41	531.51	2.12	----	----	----	----	----	----	----	----	2.12
420	2.58	531.53	2.28	----	----	----	----	----	----	----	----	2.28
440	2.74	531.55	2.44	----	----	----	----	----	----	----	----	2.44
460	2.91	531.56	2.61	----	----	----	----	----	----	----	----	2.61
480	3.07	531.58	2.78	----	----	----	----	----	----	----	----	2.78
500	3.31	531.60	2.96	----	----	----	----	----	----	----	----	2.96
520	3.74	531.63	3.22	----	----	----	----	----	----	----	----	3.22
540	4.23	531.66	3.58	----	----	----	----	----	----	----	----	3.58
560	4.67	531.71	4.00	----	----	----	----	----	----	----	----	4.00
580	4.90	531.74	4.36	----	----	----	----	----	----	----	----	4.36
600	5.43	531.78	4.73	----	----	----	----	----	----	----	----	4.73
620	6.35	531.83	5.27	----	----	----	----	----	----	----	----	5.27
640	7.59	531.91	6.05	----	----	----	----	----	----	----	----	6.05
660	9.37	532.00	6.89	----	----	----	----	----	----	----	----	6.89
680	11.90	532.04	7.15	----	----	----	----	----	----	----	----	7.15
700	21.99	532.11	7.72	----	----	----	----	----	----	----	----	7.72
720	114.94	532.57	9.77	----	----	----	----	----	----	----	----	9.77
740	87.98	533.23	13.10	----	----	----	----	----	----	----	----	13.10
760	21.92	533.35	13.64	----	----	----	----	----	----	----	----	13.64
780	14.66	533.37	13.71	----	----	----	----	----	----	----	----	13.71
800	11.75	533.37	13.70	----	----	----	----	----	----	----	----	13.70
820	9.94	533.36	13.65	----	----	----	----	----	----	----	----	13.65
840	8.58	533.34	13.58	----	----	----	----	----	----	----	----	13.58
860	7.59	533.32	13.50	----	----	----	----	----	----	----	----	13.50
880	7.07	533.30	13.40	----	----	----	----	----	----	----	----	13.40
900	6.67	533.27	13.30	----	----	----	----	----	----	----	----	13.30
920	6.26	533.25	13.19	----	----	----	----	----	----	----	----	13.19
940	5.85	533.22	13.08	----	----	----	----	----	----	----	----	13.08
960	5.44	533.20	12.96	----	----	----	----	----	----	----	----	12.96
980	5.09	533.17	12.83	----	----	----	----	----	----	----	----	12.83
1000	4.91	533.14	12.71	----	----	----	----	----	----	----	----	12.71
1020	4.76	533.11	12.58	----	----	----	----	----	----	----	----	12.58
1040	4.62	533.08	12.45	----	----	----	----	----	----	----	----	12.45

Continues on next page...

Hydrograph Discharge Table

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
1060	4.47	533.06	12.31	----	----	----	----	----	----	----	----	12.31
1080	4.32	533.03	12.18	----	----	----	----	----	----	----	----	12.18
1100	4.18	533.00	12.03	----	----	----	----	----	----	----	----	12.03
1120	4.03	532.92	11.68	----	----	----	----	----	----	----	----	11.68
1140	3.88	532.85	11.32	----	----	----	----	----	----	----	----	11.32
1160	3.74	532.79	10.97	----	----	----	----	----	----	----	----	10.97
1180	3.59	532.72	10.61	----	----	----	----	----	----	----	----	10.61
1200	3.44	532.66	10.26	----	----	----	----	----	----	----	----	10.26
1220	3.32	532.59	9.90	----	----	----	----	----	----	----	----	9.90
1240	3.28	532.53	9.54	----	----	----	----	----	----	----	----	9.54
1260	3.25	532.48	9.19	----	----	----	----	----	----	----	----	9.19
1280	3.22	532.42	8.85	----	----	----	----	----	----	----	----	8.85
1300	3.19	532.37	8.67	----	----	----	----	----	----	----	----	8.67
1320	3.16	532.32	8.62	----	----	----	----	----	----	----	----	8.62
1340	3.13	532.27	8.52	----	----	----	----	----	----	----	----	8.52
1360	3.10	532.22	8.37	----	----	----	----	----	----	----	----	8.37
1380	3.07	532.17	8.14	----	----	----	----	----	----	----	----	8.14
1400	3.04	532.13	7.85	----	----	----	----	----	----	----	----	7.85
1420	3.01	532.09	7.55	----	----	----	----	----	----	----	----	7.55
1440	2.98	532.05	7.22	----	----	----	----	----	----	----	----	7.22
1460	0.77	532.00	6.87	----	----	----	----	----	----	----	----	6.87
1480	0.00	531.70	3.94	----	----	----	----	----	----	----	----	3.94
1500	0.00	531.52	2.19	----	----	----	----	----	----	----	----	2.19

...End

Reservoir Report

Reservoir No. 3 - Main Ash Pond Pond Data

Hydraflow Hydrographs by Intelisolve

Pond storage is based on known contour areas. Average end area method used.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (acft)	Total storage (acft)
0.00	531.10	15,000	0.000	0.000
0.90	532.00	24,000	0.403	0.403
1.90	533.00	235,000	2.973	3.376
2.90	534.00	427,000	7.599	10.975

Culvert / Orifice Structures

Weir Structures

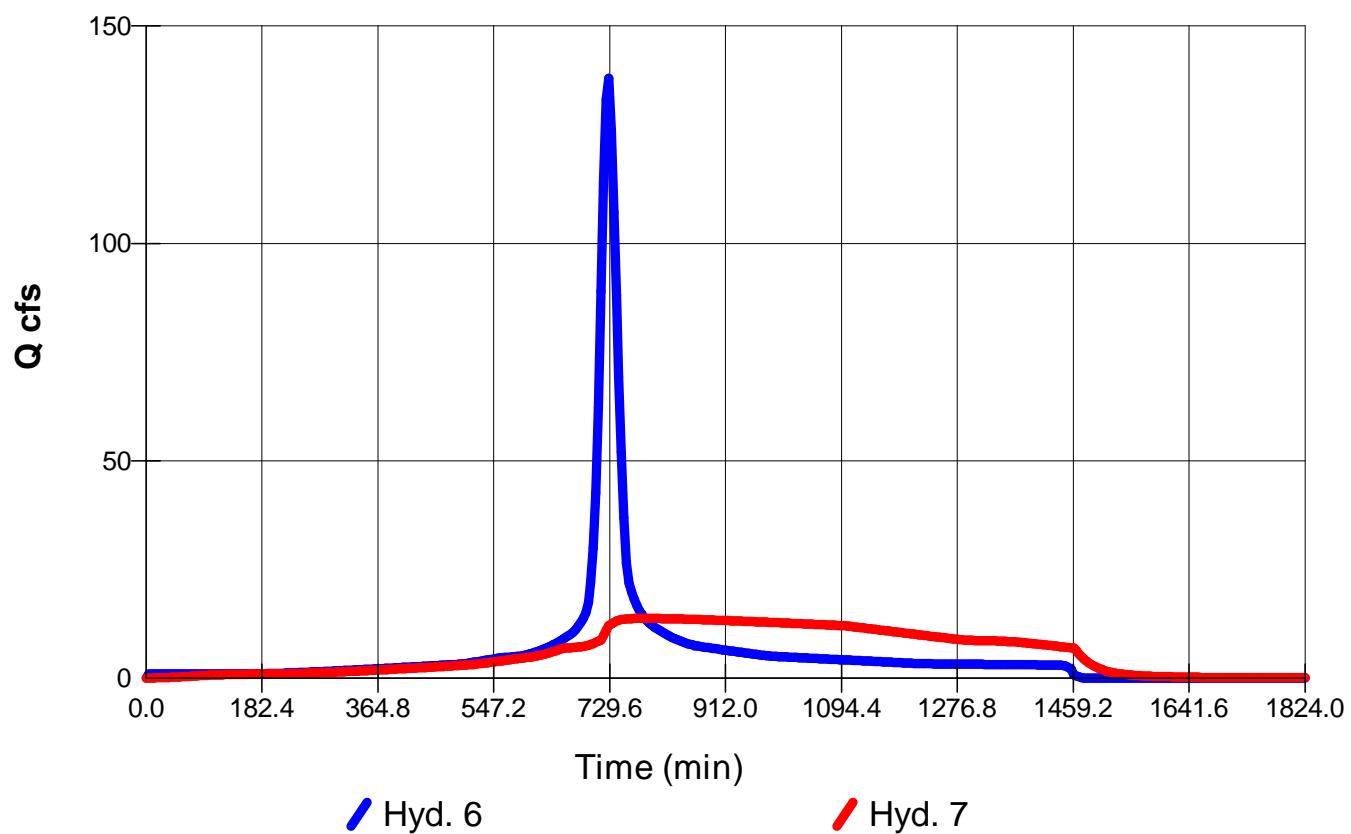
	[A]	[B]	[C]	[D]		[A]	[B]	[C]	[D]
Rise in	= 15.0	0.0	0.0	0.0	Crest Len ft	= 0.00	0.00	0.00	0.00
Span in	= 15.0	0.0	0.0	0.0	Crest El. ft	= 0.00	0.00	0.00	0.00
No. Barrels	= 2	0	0	0	Weir Coeff.	= 0.00	0.00	0.00	0.00
Invert El. ft	= 531.10	0.00	0.00	0.00	Weir Type	= ---	---	---	---
Length ft	= 60.0	0.0	0.0	0.0	Multi-Stage	= No	No	No	No
Slope %	= 1.00	0.00	0.00	0.00					
N-Value	= .015	.000	.000	.000					
Orif. Coeff.	= 0.86	0.00	0.00	0.00					
Multi-Stage	= n/a	No	No	No	Exfiltration Rate = 0.00 in/hr/sqft Tailwater Elev. = 0.00 ft				

Exfiltration Rate = 0.00 in/hr/sqft Tailwater Elev. = 0.00 ft

Stage / Storage / Discharge Table

Note: All outflows have been analyzed under inlet and outlet control.

Hyd. No. 7 - Reservoir - 1000 Yr - $Q_p = 13.71 \text{ cfs}$ - Main Ash Pond
Outflow



Hydrograph Report

Page 1

Hydraflow Hydrographs by Intelisolve

Hyd. No. 8

Economizer Area Runoff

Hydrograph type	= SCS Runoff	Peak discharge	= 237.81 cfs
Storm frequency	= 1000 yrs	Time interval	= 4 min
Drainage area	= 25.00 ac	Curve number	= 87
Basin Slope	= 1.0 %	Hydraulic length	= 700 ft
Tc method	= LAG	Time of conc. (Tc)	= 18.8 min
Total precip.	= 10.30 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

Hydrograph Volume = 18.127 acft

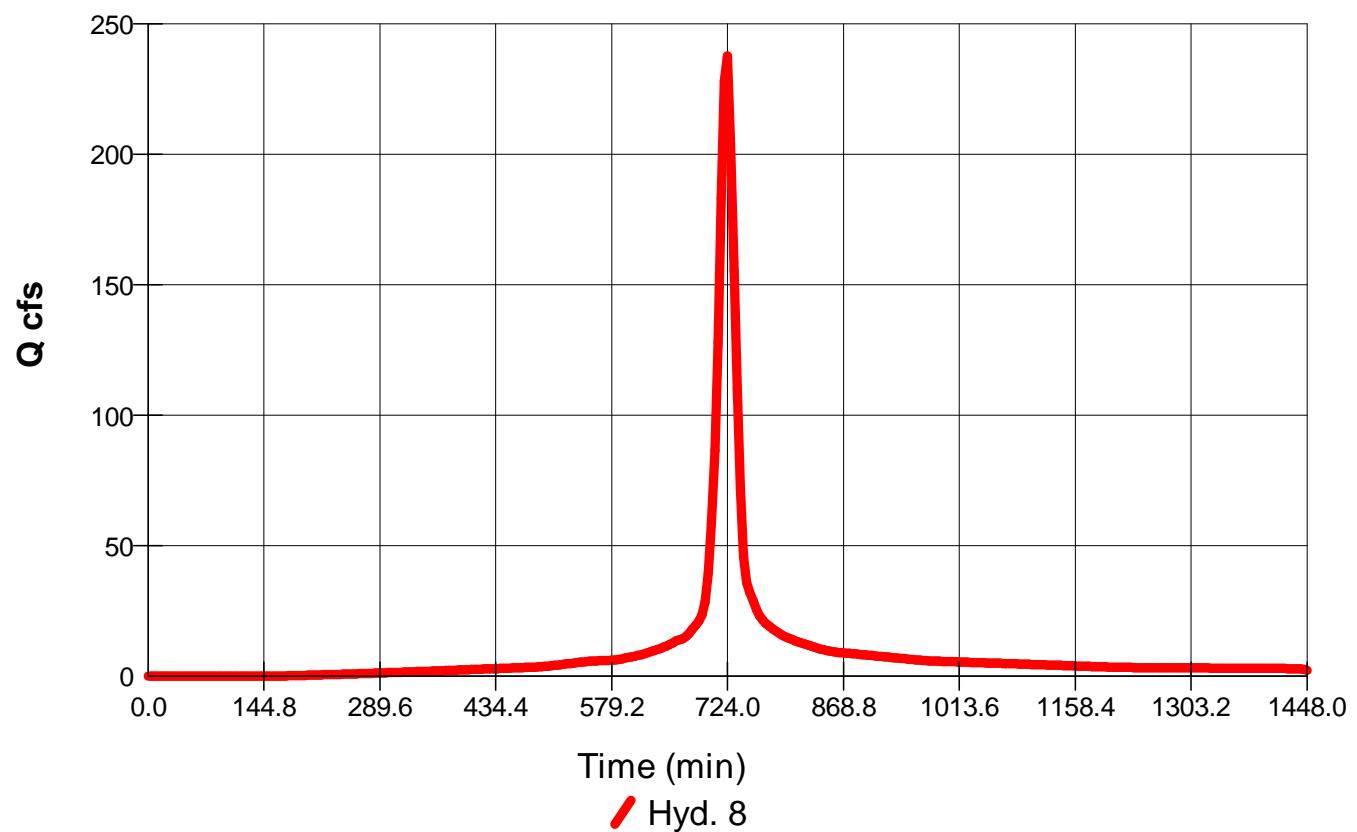
Hydrograph Discharge Table

Time -- Outflow
(min cfs)

696	28.55
716	183.34
736	110.20
756	28.60

...End

Hyd. No. 8 - SCS Runoff - 1000 Yr - $Q_p = 237.81 \text{ cfs}$ - Economizer Area
Runoff



Hydrograph Report

Page 1

Hydraflow Hydrographs by Intelisolve

Hyd. No. 9

Upper Ash Pond Inflow

Hydrograph type = Combine
Storm frequency = 1000 yrs
Inflow hyds. = 3, 7, 8

Peak discharge = 264.17 cfs
Time interval = 4 min

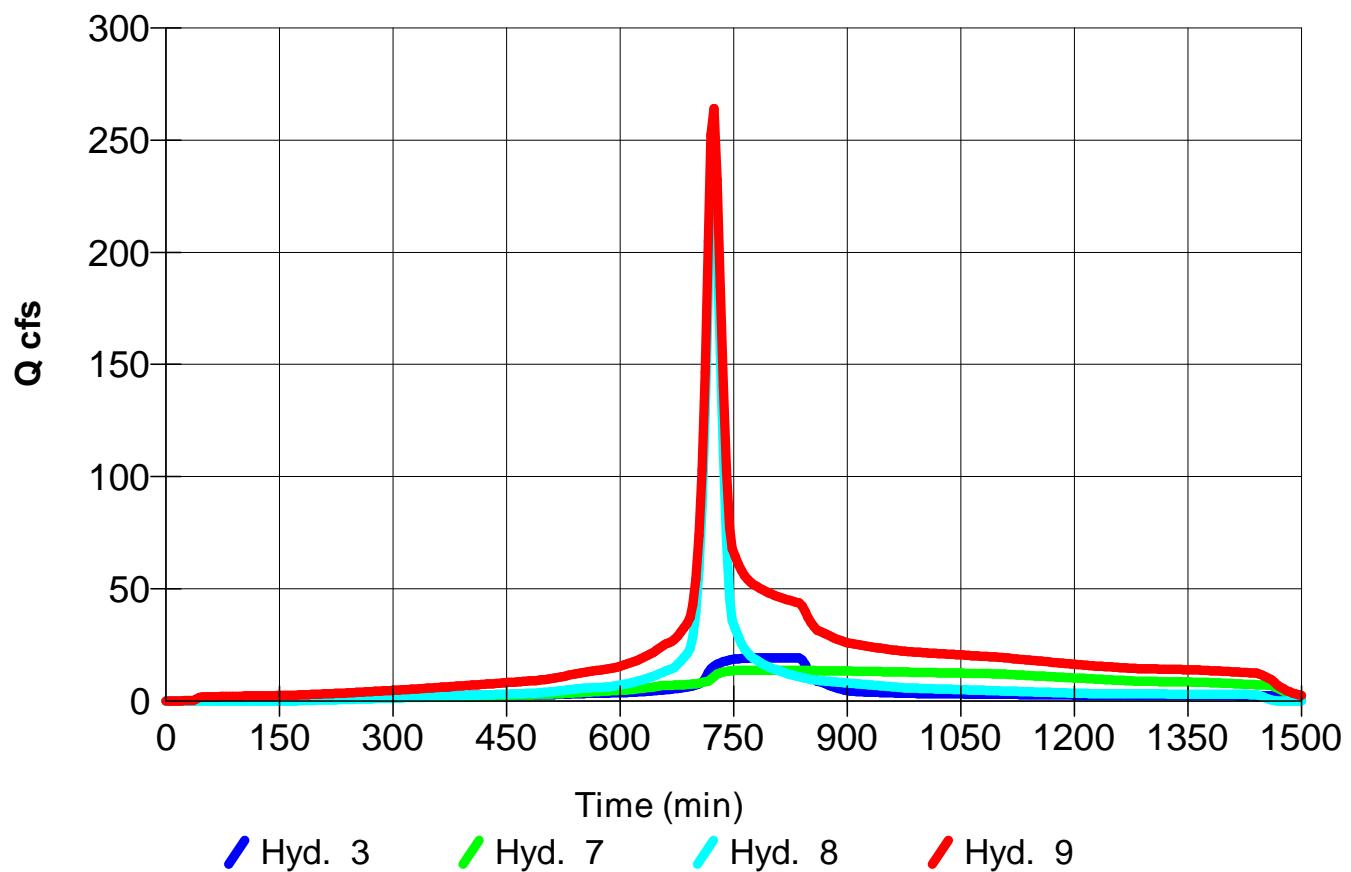
Hydrograph Volume = 41.173 acft

Hydrograph Discharge Table

Time (min)	Hyd. 3 + (cfs)	Hyd. 7 + (cfs)	Hyd. 8 = (cfs)	Outflow (cfs)
680	5.73	7.15	17.85	30.72
700	7.10	7.72	39.52	54.34
720	14.24	9.77	228.18	252.19
740	17.87	13.10	71.83	102.79
760	18.89	13.64	25.55	58.09
780	19.19	13.71	18.17	51.06
800	19.27	13.70	14.61	47.57
820	19.29	13.65	12.26	45.20
840	18.31	13.58	10.44	42.34
860	8.86	13.50	9.19	31.54
880	6.41	13.40	8.58	28.39

...End

Hyd. No. 9 - Combine - 1000 Yr - $Q_p = 264.17 \text{ cfs}$ - Upper Ash Pond Inflow



Hydrograph Report

Page 1

Hydraflow Hydrographs by Intelisolve

Hyd. No. 10

Upper Ash Pond Outflow

Hydrograph type	= Reservoir	Peak discharge	= 9.35 cfs
Storm frequency	= 1000 yrs	Time interval	= 4 min
Inflow hyd. No.	= 9	Reservoir name	= Upper Ash Pond
Max. Elevation	= 530.23 ft	Max. Storage	= 29.621 acft

Storage Indication method used.

Outflow hydrograph volume = 41.023 acft

Hydrograph Discharge Table

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
280	4.32	528.01	----	----	----	----	----	----	----	----	----	1.02
300	4.72	528.03	----	----	----	----	----	----	----	----	----	1.14
320	5.14	528.04	----	----	----	----	----	----	----	----	----	1.28
340	5.57	528.06	----	----	----	----	----	----	----	----	----	1.43
360	6.03	528.08	----	----	----	----	----	----	----	----	----	1.59
380	6.49	528.10	----	----	----	----	----	----	----	----	----	1.76
400	6.96	528.11	----	----	----	----	----	----	----	----	----	1.89
420	7.43	528.12	----	----	----	----	----	----	----	----	----	2.01
440	7.91	528.13	----	----	----	----	----	----	----	----	----	2.14
460	8.39	528.14	----	----	----	----	----	----	----	----	----	2.27
480	8.88	528.16	----	----	----	----	----	----	----	----	----	2.41
500	9.55	528.17	----	----	----	----	----	----	----	----	----	2.57
520	10.66	528.19	----	----	----	----	----	----	----	----	----	2.74
540	11.99	528.20	----	----	----	----	----	----	----	----	----	2.93
560	13.20	528.22	----	----	----	----	----	----	----	----	----	3.15
580	13.99	528.24	----	----	----	----	----	----	----	----	----	3.38
600	15.50	528.27	----	----	----	----	----	----	----	----	----	3.64
620	17.80	528.29	----	----	----	----	----	----	----	----	----	3.93
640	20.99	528.33	----	----	----	----	----	----	----	----	----	4.30
660	25.35	528.36	----	----	----	----	----	----	----	----	----	4.77
680	30.72	528.41	----	----	----	----	----	----	----	----	----	5.32
700	54.34	528.48	----	----	----	----	----	----	----	----	----	6.13
720	252.19	528.75	----	----	----	----	----	----	----	----	----	7.66
740	102.79	529.14	----	----	----	----	----	----	----	----	----	8.09
760	58.09	529.27	----	----	----	----	----	----	----	----	----	8.23
780	51.06	529.37	----	----	----	----	----	----	----	----	----	8.34
800	47.57	529.45	----	----	----	----	----	----	----	----	----	8.43
820	45.20	529.53	----	----	----	----	----	----	----	----	----	8.51
840	42.34	529.60	----	----	----	----	----	----	----	----	----	8.59
860	31.54	529.66	----	----	----	----	----	----	----	----	----	8.66
880	28.39	529.70	----	----	----	----	----	----	----	----	----	8.71
900	25.92	529.74	----	----	----	----	----	----	----	----	----	8.75
920	24.66	529.78	----	----	----	----	----	----	----	----	----	8.79
940	23.68	529.81	----	----	----	----	----	----	----	----	----	8.83
960	22.76	529.84	----	----	----	----	----	----	----	----	----	8.87
980	22.01	529.87	----	----	----	----	----	----	----	----	----	8.90
1000	21.54	529.89	----	----	----	----	----	----	----	----	----	8.94
1020	21.12	529.92	----	----	----	----	----	----	----	----	----	8.97

Continues on next page...

Hydrograph Discharge Table

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
1040	20.71	529.94	----	----	----	----	----	----	----	----	----	9.00
1060	20.30	529.97	----	----	----	----	----	----	----	----	----	9.03
1080	19.89	529.99	----	----	----	----	----	----	----	----	----	9.06
1100	19.47	530.01	----	----	----	----	----	----	----	----	----	9.08
1120	18.84	530.03	----	----	----	----	----	----	----	----	----	9.11
1140	18.21	530.05	----	----	----	----	----	----	----	----	----	9.13
1160	17.58	530.07	----	----	----	----	----	----	----	----	----	9.16
1180	16.95	530.09	----	----	----	----	----	----	----	----	----	9.18
1200	16.31	530.10	----	----	----	----	----	----	----	----	----	9.20
1220	15.75	530.12	----	----	----	----	----	----	----	----	----	9.21
1240	15.31	530.13	----	----	----	----	----	----	----	----	----	9.23
1260	14.90	530.14	----	----	----	----	----	----	----	----	----	9.24
1280	14.49	530.15	----	----	----	----	----	----	----	----	----	9.26
1300	14.26	530.16	----	----	----	----	----	----	----	----	----	9.27
1320	14.16	530.17	----	----	----	----	----	----	----	----	----	9.28
1340	14.00	530.18	----	----	----	----	----	----	----	----	----	9.30
1360	13.79	530.19	----	----	----	----	----	----	----	----	----	9.31
1380	13.51	530.20	----	----	----	----	----	----	----	----	----	9.32
1400	13.16	530.21	----	----	----	----	----	----	----	----	----	9.33
1420	12.81	530.22	----	----	----	----	----	----	----	----	----	9.34
1440	12.42	530.22	----	----	----	----	----	----	----	----	----	9.35
1460	9.48	530.23 <<	----	----	----	----	----	----	----	----	----	9.35 <<
1480	4.68	530.22	----	----	----	----	----	----	----	----	----	9.34
1500	2.40	530.21	----	----	----	----	----	----	----	----	----	9.33
1520	1.38	530.20	----	----	----	----	----	----	----	----	----	9.31
1540	0.87	530.18	----	----	----	----	----	----	----	----	----	9.29
1560	0.59	530.16	----	----	----	----	----	----	----	----	----	9.27
1580	0.42	530.14	----	----	----	----	----	----	----	----	----	9.24
1600	0.34	530.12	----	----	----	----	----	----	----	----	----	9.22
1620	0.27	530.11	----	----	----	----	----	----	----	----	----	9.20
1640	0.21	530.09	----	----	----	----	----	----	----	----	----	9.18
1660	0.17	530.07	----	----	----	----	----	----	----	----	----	9.15
1680	0.13	530.05	----	----	----	----	----	----	----	----	----	9.13
1700	0.11	530.03	----	----	----	----	----	----	----	----	----	9.11
1720	0.10	530.01	----	----	----	----	----	----	----	----	----	9.08
1740	0.09	529.99	----	----	----	----	----	----	----	----	----	9.06
1760	0.09	529.98	----	----	----	----	----	----	----	----	----	9.04
1780	0.08	529.96	----	----	----	----	----	----	----	----	----	9.02
1800	0.08	529.94	----	----	----	----	----	----	----	----	----	8.99
1820	0.07	529.92	----	----	----	----	----	----	----	----	----	8.97
1840	0.06	529.90	----	----	----	----	----	----	----	----	----	8.95
1860	0.06	529.88	----	----	----	----	----	----	----	----	----	8.93
1880	0.06	529.86	----	----	----	----	----	----	----	----	----	8.90
1900	0.05	529.85	----	----	----	----	----	----	----	----	----	8.88
1920	0.05	529.83	----	----	----	----	----	----	----	----	----	8.86
1940	0.04	529.81	----	----	----	----	----	----	----	----	----	8.84
1960	0.04	529.79	----	----	----	----	----	----	----	----	----	8.81
1980	0.04	529.77	----	----	----	----	----	----	----	----	----	8.79
2000	0.03	529.76	----	----	----	----	----	----	----	----	----	8.77
2020	0.03	529.74	----	----	----	----	----	----	----	----	----	8.75
2040	0.03	529.72	----	----	----	----	----	----	----	----	----	8.72

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Hydrograph Discharge Table

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
2060	0.03	529.70	----	----	----	----	----	----	----	----	----	8.70
2080	0.03	529.68	----	----	----	----	----	----	----	----	----	8.68
2100	0.02	529.67	----	----	----	----	----	----	----	----	----	8.66
2120	0.02	529.65	----	----	----	----	----	----	----	----	----	8.64
2140	0.02	529.63	----	----	----	----	----	----	----	----	----	8.62
2160	0.02	529.61	----	----	----	----	----	----	----	----	----	8.60
2180	0.02	529.59	----	----	----	----	----	----	----	----	----	8.58
2200	0.02	529.58	----	----	----	----	----	----	----	----	----	8.57
2220	0.01	529.56	----	----	----	----	----	----	----	----	----	8.55
2240	0.01	529.54	----	----	----	----	----	----	----	----	----	8.53
2260	0.01	529.52	----	----	----	----	----	----	----	----	----	8.51
2280	0.01	529.51	----	----	----	----	----	----	----	----	----	8.49
2300	0.01	529.49	----	----	----	----	----	----	----	----	----	8.47
2320	0.01	529.47	----	----	----	----	----	----	----	----	----	8.45
2340	0.01	529.45	----	----	----	----	----	----	----	----	----	8.43
2360	0.01	529.44	----	----	----	----	----	----	----	----	----	8.41
2380	0.01	529.42	----	----	----	----	----	----	----	----	----	8.39
2400	0.01	529.40	----	----	----	----	----	----	----	----	----	8.37
2420	0.01	529.38	----	----	----	----	----	----	----	----	----	8.36
2440	0.01	529.37	----	----	----	----	----	----	----	----	----	8.34
2460	0.01	529.35	----	----	----	----	----	----	----	----	----	8.32
2480	0.01	529.33	----	----	----	----	----	----	----	----	----	8.30
2500	0.01	529.31	----	----	----	----	----	----	----	----	----	8.28
2520	0.00	529.30	----	----	----	----	----	----	----	----	----	8.26
2540	0.00	529.28	----	----	----	----	----	----	----	----	----	8.24
2560	0.00	529.26	----	----	----	----	----	----	----	----	----	8.22
2580	0.00	529.25	----	----	----	----	----	----	----	----	----	8.21
2600	0.00	529.23	----	----	----	----	----	----	----	----	----	8.19
2620	0.00	529.21	----	----	----	----	----	----	----	----	----	8.17
2640	0.00	529.20	----	----	----	----	----	----	----	----	----	8.15
2660	0.00	529.18	----	----	----	----	----	----	----	----	----	8.13
2680	0.00	529.16	----	----	----	----	----	----	----	----	----	8.11
2700	0.00	529.15	----	----	----	----	----	----	----	----	----	8.09
2720	0.00	529.13	----	----	----	----	----	----	----	----	----	8.08
2740	0.00	529.11	----	----	----	----	----	----	----	----	----	8.06
2760	0.00	529.10	----	----	----	----	----	----	----	----	----	8.04
2780	0.00	529.08	----	----	----	----	----	----	----	----	----	8.02
2800	0.00	529.06	----	----	----	----	----	----	----	----	----	8.00
2820	0.00	529.05	----	----	----	----	----	----	----	----	----	7.99
2840	0.00	529.03	----	----	----	----	----	----	----	----	----	7.97
2860	0.00	529.01	----	----	----	----	----	----	----	----	----	7.95
2880	0.00	529.00	----	----	----	----	----	----	----	----	----	7.93
2900	0.00	528.98	----	----	----	----	----	----	----	----	----	7.91
2920	0.00	528.96	----	----	----	----	----	----	----	----	----	7.90
2940	0.00	528.95	----	----	----	----	----	----	----	----	----	7.88
2960	0.00	528.93	----	----	----	----	----	----	----	----	----	7.86
2980	0.00	528.91	----	----	----	----	----	----	----	----	----	7.84
3000	0.00	528.90	----	----	----	----	----	----	----	----	----	7.83
3020	0.00	528.88	----	----	----	----	----	----	----	----	----	7.81
3040	0.00	528.87	----	----	----	----	----	----	----	----	----	7.79
3060	0.00	528.85	----	----	----	----	----	----	----	----	----	7.77

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Hydrograph Discharge Table

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
3080	0.00	528.83	----	----	----	----	----	----	----	----	----	7.75
3100	0.00	528.82	----	----	----	----	----	----	----	----	----	7.74
3120	0.00	528.80	----	----	----	----	----	----	----	----	----	7.72
3140	0.00	528.79	----	----	----	----	----	----	----	----	----	7.70
3160	0.00	528.77	----	----	----	----	----	----	----	----	----	7.68
3180	0.00	528.75	----	----	----	----	----	----	----	----	----	7.67
3200	0.00	528.74	----	----	----	----	----	----	----	----	----	7.65
3220	0.00	528.72	----	----	----	----	----	----	----	----	----	7.63
3240	0.00	528.71	----	----	----	----	----	----	----	----	----	7.62
3260	0.00	528.69	----	----	----	----	----	----	----	----	----	7.60
3280	0.00	528.67	----	----	----	----	----	----	----	----	----	7.58
3300	0.00	528.66	----	----	----	----	----	----	----	----	----	7.56
3320	0.00	528.64	----	----	----	----	----	----	----	----	----	7.55
3340	0.00	528.63	----	----	----	----	----	----	----	----	----	7.53
3360	0.00	528.61	----	----	----	----	----	----	----	----	----	7.51
3380	0.00	528.60	----	----	----	----	----	----	----	----	----	7.46
3400	0.00	528.58	----	----	----	----	----	----	----	----	----	7.29
3420	0.00	528.57	----	----	----	----	----	----	----	----	----	7.13
3440	0.00	528.55	----	----	----	----	----	----	----	----	----	6.97
3460	0.00	528.54	----	----	----	----	----	----	----	----	----	6.81
3480	0.00	528.52	----	----	----	----	----	----	----	----	----	6.66
3500	0.00	528.51	----	----	----	----	----	----	----	----	----	6.51
3520	0.00	528.50	----	----	----	----	----	----	----	----	----	6.36
3540	0.00	528.48	----	----	----	----	----	----	----	----	----	6.20
3560	0.00	528.47	----	----	----	----	----	----	----	----	----	6.05
3580	0.00	528.46	----	----	----	----	----	----	----	----	----	5.90
3600	0.00	528.45	----	----	----	----	----	----	----	----	----	5.75
3620	0.00	528.43	----	----	----	----	----	----	----	----	----	5.61
3640	0.00	528.42	----	----	----	----	----	----	----	----	----	5.47
3660	0.00	528.41	----	----	----	----	----	----	----	----	----	5.34
3680	0.00	528.40	----	----	----	----	----	----	----	----	----	5.21
3700	0.00	528.39	----	----	----	----	----	----	----	----	----	5.08
3720	0.00	528.38	----	----	----	----	----	----	----	----	----	4.96
3740	0.00	528.37	----	----	----	----	----	----	----	----	----	4.83
3760	0.00	528.36	----	----	----	----	----	----	----	----	----	4.72
3780	0.00	528.35	----	----	----	----	----	----	----	----	----	4.60
3800	0.00	528.34	----	----	----	----	----	----	----	----	----	4.49
3820	0.00	528.33	----	----	----	----	----	----	----	----	----	4.38
3840	0.00	528.32	----	----	----	----	----	----	----	----	----	4.27
3860	0.00	528.31	----	----	----	----	----	----	----	----	----	4.16
3880	0.00	528.31	----	----	----	----	----	----	----	----	----	4.06
3900	0.00	528.30	----	----	----	----	----	----	----	----	----	3.97
3920	0.00	528.29	----	----	----	----	----	----	----	----	----	3.88
3940	0.00	528.28	----	----	----	----	----	----	----	----	----	3.79
3960	0.00	528.27	----	----	----	----	----	----	----	----	----	3.70
3980	0.00	528.27	----	----	----	----	----	----	----	----	----	3.62
4000	0.00	528.26	----	----	----	----	----	----	----	----	----	3.54
4020	0.00	528.25	----	----	----	----	----	----	----	----	----	3.46
4040	0.00	528.24	----	----	----	----	----	----	----	----	----	3.38
4060	0.00	528.24	----	----	----	----	----	----	----	----	----	3.30
4080	0.00	528.23	----	----	----	----	----	----	----	----	----	3.23

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Hydrograph Discharge Table

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
4100	0.00	528.22	----	----	----	----	----	----	----	----	----	3.16
4120	0.00	528.22	----	----	----	----	----	----	----	----	----	3.09
4140	0.00	528.21	----	----	----	----	----	----	----	----	----	3.02
4160	0.00	528.20	----	----	----	----	----	----	----	----	----	2.95
4180	0.00	528.20	----	----	----	----	----	----	----	----	----	2.88
4200	0.00	528.19	----	----	----	----	----	----	----	----	----	2.82
4220	0.00	528.19	----	----	----	----	----	----	----	----	----	2.75
4240	0.00	528.18	----	----	----	----	----	----	----	----	----	2.69
4260	0.00	528.18	----	----	----	----	----	----	----	----	----	2.63
4280	0.00	528.17	----	----	----	----	----	----	----	----	----	2.57
4300	0.00	528.16	----	----	----	----	----	----	----	----	----	2.51
4320	0.00	528.16	----	----	----	----	----	----	----	----	----	2.46
4340	0.00	528.15	----	----	----	----	----	----	----	----	----	2.40
4360	0.00	528.15	----	----	----	----	----	----	----	----	----	2.35
4380	0.00	528.15	----	----	----	----	----	----	----	----	----	2.29
4400	0.00	528.14	----	----	----	----	----	----	----	----	----	2.24
4420	0.00	528.14	----	----	----	----	----	----	----	----	----	2.19
4440	0.00	528.13	----	----	----	----	----	----	----	----	----	2.14
4460	0.00	528.13	----	----	----	----	----	----	----	----	----	2.09
4480	0.00	528.12	----	----	----	----	----	----	----	----	----	2.05
4500	0.00	528.12	----	----	----	----	----	----	----	----	----	2.00
4520	0.00	528.11	----	----	----	----	----	----	----	----	----	1.96
4540	0.00	528.11	----	----	----	----	----	----	----	----	----	1.91
4560	0.00	528.11	----	----	----	----	----	----	----	----	----	1.87
4580	0.00	528.10	----	----	----	----	----	----	----	----	----	1.83
4600	0.00	528.10	----	----	----	----	----	----	----	----	----	1.78
4620	0.00	528.09	----	----	----	----	----	----	----	----	----	1.71
4640	0.00	528.08	----	----	----	----	----	----	----	----	----	1.65
4660	0.00	528.08	----	----	----	----	----	----	----	----	----	1.59
4680	0.00	528.07	----	----	----	----	----	----	----	----	----	1.53
4700	0.00	528.06	----	----	----	----	----	----	----	----	----	1.47
4720	0.00	528.06	----	----	----	----	----	----	----	----	----	1.42
4740	0.00	528.05	----	----	----	----	----	----	----	----	----	1.37
4760	0.00	528.05	----	----	----	----	----	----	----	----	----	1.32
4780	0.00	528.04	----	----	----	----	----	----	----	----	----	1.27
4800	0.00	528.04	----	----	----	----	----	----	----	----	----	1.22
4820	0.00	528.03	----	----	----	----	----	----	----	----	----	1.18
4840	0.00	528.03	----	----	----	----	----	----	----	----	----	1.14
4860	0.00	528.02	----	----	----	----	----	----	----	----	----	1.09
4880	0.00	528.02	----	----	----	----	----	----	----	----	----	1.05
4900	0.00	528.01	----	----	----	----	----	----	----	----	----	1.02
4920	0.00	528.01	----	----	----	----	----	----	----	----	----	0.98
4940	0.00	528.00	----	----	----	----	----	----	----	----	----	0.94

...End

Reservoir Report

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Reservoir No. 2 - Upper Ash Pond

Hydraflow Hydrographs by InteliSolve

Pond Data

Pond storage is based on known contour areas. Average end area method used.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (acft)	Total storage (acft)
0.00	527.90	00	0.000	0.000
0.20	528.10	579,000	1.329	1.329
0.40	528.30	579,000	2.658	3.988
0.60	528.50	579,000	2.658	6.646
0.70	528.60	579,000	1.329	7.975
1.80	529.70	579,000	14.621	22.596
3.10	531.00	579,000	17.280	39.876
4.00	531.90	579,000	11.963	51.839

Culvert / Orifice Structures

Weir Structures

	[A]	[B]	[C]	[D]		[A]	[B]	[C]	[D]
Rise in	= 0.0	0.0	0.0	0.0	Crest Len ft	= 0.00	0.00	0.00	0.00
Span in	= 0.0	0.0	0.0	0.0	Crest El. ft	= 0.00	0.00	0.00	0.00
No. Barrels	= 0	0	0	0	Weir Coeff.	= 0.00	0.00	0.00	0.00
Invert El. ft	= 0.00	0.00	0.00	0.00	Weir Type	= ---	---	---	---
Length ft	= 0.0	0.0	0.0	0.0	Multi-Stage	= No	No	No	No
Slope %	= 0.00	0.00	0.00	0.00					
N-Value	= .013	.000	.000	.000					
Orif. Coeff.	= 0.60	0.00	0.00	0.00					
Multi-Stage	= n/a	No	No	No					

Exfiltration Rate = 0.00 in/hr/sqft Tailwater Elev. = 0.00 ft

Note: All outflows have been analyzed under inlet and outlet control.

Stage / Storage / Discharge Table

Stage ft	Storage acft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Total cfs
0.00	0.000	527.90	---	---	---	---	---	---	---	---	---	0.00
0.02	0.133	527.92	---	---	---	---	---	---	---	---	---	0.18
0.04	0.266	527.94	---	---	---	---	---	---	---	---	---	0.36
0.06	0.399	527.96	---	---	---	---	---	---	---	---	---	0.54
0.08	0.532	527.98	---	---	---	---	---	---	---	---	---	0.72
0.10	0.665	528.00	---	---	---	---	---	---	---	---	---	0.90
0.12	0.798	528.02	---	---	---	---	---	---	---	---	---	1.08
0.14	0.930	528.04	---	---	---	---	---	---	---	---	---	1.26
0.16	1.063	528.06	---	---	---	---	---	---	---	---	---	1.44
0.18	1.196	528.08	---	---	---	---	---	---	---	---	---	1.62
0.20	1.329	528.10	---	---	---	---	---	---	---	---	---	1.80
0.22	1.595	528.12	---	---	---	---	---	---	---	---	---	2.02
0.24	1.861	528.14	---	---	---	---	---	---	---	---	---	2.24
0.26	2.127	528.16	---	---	---	---	---	---	---	---	---	2.46
0.28	2.393	528.18	---	---	---	---	---	---	---	---	---	2.68
0.30	2.658	528.20	---	---	---	---	---	---	---	---	---	2.90
0.32	2.924	528.22	---	---	---	---	---	---	---	---	---	3.12
0.34	3.190	528.24	---	---	---	---	---	---	---	---	---	3.34
0.36	3.456	528.26	---	---	---	---	---	---	---	---	---	3.56
0.38	3.722	528.28	---	---	---	---	---	---	---	---	---	3.78
0.40	3.988	528.30	---	---	---	---	---	---	---	---	---	4.00
0.42	4.253	528.32	---	---	---	---	---	---	---	---	---	4.24
0.44	4.519	528.34	---	---	---	---	---	---	---	---	---	4.48
0.46	4.785	528.36	---	---	---	---	---	---	---	---	---	4.72
0.48	5.051	528.38	---	---	---	---	---	---	---	---	---	4.96
0.50	5.317	528.40	---	---	---	---	---	---	---	---	---	5.20
0.52	5.583	528.42	---	---	---	---	---	---	---	---	---	5.44
0.54	5.848	528.44	---	---	---	---	---	---	---	---	---	5.68
0.56	6.114	528.46	---	---	---	---	---	---	---	---	---	5.92
0.58	6.380	528.48	---	---	---	---	---	---	---	---	---	6.16

Continues on next page...

Upper Ash Pond
Stage / Storage / Discharge Table

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Stage ft	Storage acft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Total cfs
0.60	6.646	528.50	---	---	---	---	---	---	---	---	---	6.40
0.61	6.779	528.51	---	---	---	---	---	---	---	---	---	6.51
0.62	6.912	528.52	---	---	---	---	---	---	---	---	---	6.62
0.63	7.045	528.53	---	---	---	---	---	---	---	---	---	6.73
0.64	7.178	528.54	---	---	---	---	---	---	---	---	---	6.84
0.65	7.311	528.55	---	---	---	---	---	---	---	---	---	6.95
0.66	7.444	528.56	---	---	---	---	---	---	---	---	---	7.06
0.67	7.576	528.57	---	---	---	---	---	---	---	---	---	7.17
0.68	7.709	528.58	---	---	---	---	---	---	---	---	---	7.28
0.69	7.842	528.59	---	---	---	---	---	---	---	---	---	7.39
0.70	7.975	528.60	---	---	---	---	---	---	---	---	---	7.50
0.81	9.437	528.71	---	---	---	---	---	---	---	---	---	7.62
0.92	10.899	528.82	---	---	---	---	---	---	---	---	---	7.74
1.03	12.362	528.93	---	---	---	---	---	---	---	---	---	7.86
1.14	13.824	529.04	---	---	---	---	---	---	---	---	---	7.98
1.25	15.286	529.15	---	---	---	---	---	---	---	---	---	8.10
1.36	16.748	529.26	---	---	---	---	---	---	---	---	---	8.22
1.47	18.210	529.37	---	---	---	---	---	---	---	---	---	8.34
1.58	19.672	529.48	---	---	---	---	---	---	---	---	---	8.46
1.69	21.134	529.59	---	---	---	---	---	---	---	---	---	8.58
1.80	22.596	529.70	---	---	---	---	---	---	---	---	---	8.70
1.93	24.324	529.83	---	---	---	---	---	---	---	---	---	8.86
2.06	26.052	529.96	---	---	---	---	---	---	---	---	---	9.02
2.19	27.780	530.09	---	---	---	---	---	---	---	---	---	9.18
2.32	29.508	530.22	---	---	---	---	---	---	---	---	---	9.34
2.45	31.236	530.35	---	---	---	---	---	---	---	---	---	9.50
2.58	32.964	530.48	---	---	---	---	---	---	---	---	---	9.66
2.71	34.692	530.61	---	---	---	---	---	---	---	---	---	9.82
2.84	36.420	530.74	---	---	---	---	---	---	---	---	---	9.98
2.97	38.148	530.87	---	---	---	---	---	---	---	---	---	10.14
3.10	39.876	531.00	---	---	---	---	---	---	---	---	---	10.30
3.19	41.072	531.09	---	---	---	---	---	---	---	---	---	10.37
3.28	42.269	531.18	---	---	---	---	---	---	---	---	---	10.44
3.37	43.465	531.27	---	---	---	---	---	---	---	---	---	10.51
3.46	44.661	531.36	---	---	---	---	---	---	---	---	---	10.58
3.55	45.857	531.45	---	---	---	---	---	---	---	---	---	10.65
3.64	47.054	531.54	---	---	---	---	---	---	---	---	---	10.72
3.73	48.250	531.63	---	---	---	---	---	---	---	---	---	10.79
3.82	49.446	531.72	---	---	---	---	---	---	---	---	---	10.86
3.91	50.643	531.81	---	---	---	---	---	---	---	---	---	10.93
4.00	51.839	531.90	---	---	---	---	---	---	---	---	---	11.00

...End

Hyd. No. 10 - Reservoir - 1000 Yr - $Q_p = 9.35 \text{ cfs}$ - Upper Ash Pond
Outflow

