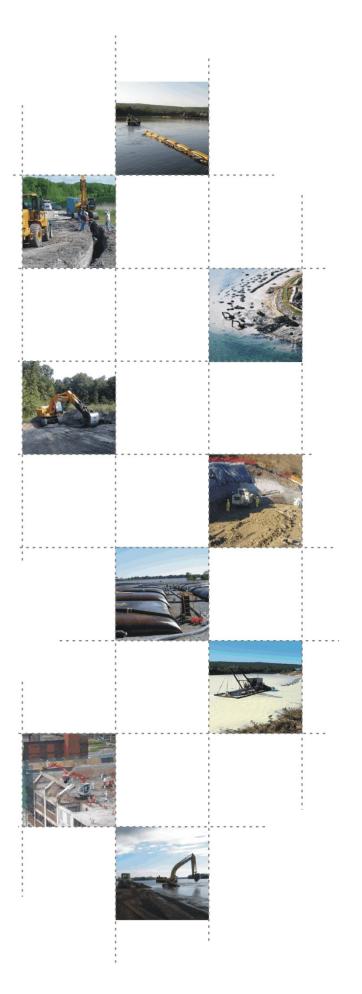
ALLIANT ENERGY Interstate Power and Light Company Burlington Generating Station

CCR SURFACE IMPOUNDMENT

INFLOW DESIGN FLOOD CONTROL PLAN

Report Issued: June 21, 2021 Revision 2





EXECUTIVE SUMMARY

This Inflow Flood Control Plan (Report) is prepared in accordance with the requirements of the United States Environmental Protection Agency (USEPA) published Final Rule for Hazardous and Solid Waste Management System – Disposal of Coal Combustion Residual from Electric Utilities (40 CFR Parts 257 and 261, also known as the CCR Rule) published on April 17, 2015 (effective October 19, 2015) and subsequent amendments.

This Report assesses the hydrologic and hydraulic capacity requirements for each CCR unit at Burlington Generating Station in Burlington, Iowa in accordance with §257.82 of the CCR Rule. For purposes of this Report, a CCR unit is defined as any existing CCR surface impoundment. Primarily, the Report documents how the inflow design flood control system has been designed and constructed to meet the CCR Rule section §257.82.



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1 INTRODUCTION

The owner or operator of the Coal Combustion Residual (CCR) unit must conduct an initial and periodic inflow design flood control system plan to determine if each CCR unit adequately manages flow into and from each CCR unit during and following the peak discharge of the inflow design flood. The inflow design flood is selected based on the hazard potential classification (§257.73(a)(2)) for each CCR unit.

Revision 2 of this Report has been prepared in accordance with the requirements of \$257.82 of the CCR Rule.

1.1 CCR Rule Applicability

The CCR Rule requires an initial and periodic inflow design flood control system plan certified by a qualified professional engineer (PE) for all existing CCR surface impoundments.

1.2 Hydrologic and Hydraulic Capacity Applicability

The Burlington Generating Station (BGS) in Burlington, Iowa (Figure 1) has four existing CCR surface impoundments that meet the requirements of §257.73(b)(1) or §257.73(b)(2) of the CCR Rule, which are identified as follows:

- BGS Ash Seal Pond
- BGS Main Ash Pond
- BGS Economizer Pond
- BGS Upper Ash Pond



2 FACILITY DESCRIPTION

The following sub-section provides a summary description of the facility and existing CCR surface impoundments located at BGS.

BGS is located southeast of the City of Burlington, Iowa on the western shore of the Mississippi River in Des Moines County, at 4282 Sullivan Slough Road, Burlington, Iowa (Figure 1). BGS is a fossil-fueled electric generating station consisting of one steam electric generating unit and four combustion turbine units. Sub-bituminous coal is the primary fuel for producing steam, with the ability to use natural gas for the combustion turbines. The burning of coal produces a by-product of CCR. The CCR at BGS is categorized into three types: bottom ash, economizer ash, and precipitator fly ash.

General Facility Information:

Date of Initial Facility Operations: 1968						
NPDES Permit Number:	IA29-00-1-01					
Facility Title V Operating Permit	: 98-TV-023R1-M004					
Latitude / Longitude:	40°44′29″N 91°07′04″W					
Site Coordinates:	Section 29, Township 69 North, Range 02 West					
Unit Nameplate Ratings:	Unit 1: 212 MW					

2.1 BGS Ash Seal Pond

The BGS Ash Seal Pond is located south of the generating plant and east of the BGS Main Ash Pond. The CCR, in 1968, was originally managed by discharging into the BGS Ash Seal Pond for settling. Presently, the BGS Ash Seal Pond only receives storm water runoff from the surrounding area associated with the fly ash storage silo. The BGS Ash Seal Pond also may receive facility process water, such as ash seal water, but only if there is an issue with the ash seal water pumps. At the time of the last annual inspection on June 3, 2020 the CCR surface impoundment did not contain standing water.



The surface area of the BGS Ash Seal Pond is approximately 5.7 acres and has an embankment height of approximately 12 feet from the crest to the toe of the downstream slope. The embankment crest is at elevation 534 the same as the adjacent plant site grade and equivalent to the 100-year flood water elevation of the Mississippi River. The interior storage depth of the BGS Ash Seal Pond is approximately 12 feet. As stated in the 2020 Annual Inspection, the total volume of impounded CCR and water within the BGS Ash Seal Pond is approximately 106,000 cubic yards, which would include general fill that has been added in the northeast corner of the impoundment. The original outfall for the impoundment is sealed to prevent discharge to the Mississippi River and the impoundment normally contains no water. Rainfall that accumulates exfiltrates through the bottom of the impoundment. A manually operated pump is available to lift storm water to the adjacent BGS Main Ash Pond, if necessary.

2.2 BGS Main Ash Pond

The BGS Main Ash Pond is located southwest of the generating plant and west of the BGS Ash Seal Pond. The CCR, prior to being sluiced to the BGS Main Ash Pond, was originally managed in the BGS Ash Seal Pond in 1968. In 1971, BGS managed CCR in the BGS Upper Ash Pond. In 1980, the BGS Main Ash Pond became the primary receiver of CCR, with the BGS Upper Ash Pond becoming a downstream receiver.

Presently, the BGS Main Ash Pond receives bottom ash that is sluiced from the generating plant to the northeast corner of the BGS Main Ash Pond. The sluiced bottom ash discharges into the northeast corner where most of the bottom ash settles out. This bottom ash is recovered for beneficial reuse. Hydrated fly ash is also stored within the BGS Main Ash Pond area prior to being sold as aggregate material for beneficial reuse. Fly ash from the on-site storage silo is no longer added to the pile within impoundment.

The water that is used to sluice the bottom ash into the BGS Main Ash Pond is routed towards the west end of the BGS Main Ash Pond. The water is discharged in batch quantities as bottom ash accumulates in the boiler and averages 1 cubic foot per second



(cfs) daily. From that point, the water flows to the west along the north side of a road constructed out of bottom ash through the center of the BGS Main Ash Pond, Figure 2. Then flows along the north side of the road until it reaches the west end where it transitions into a ponded area in the northwest corner of the BGS Main Ash Pond. The water in the northwest corner of the BGS Main Ash Pond. The diameter corrugated metal culverts with identical invert elevation under the generating plant entrance road where it discharges into a small channel in the southwest corner of the BGS Upper Ash Pond located north of the generating plant entrance road.

The surface area of the BGS Main Ash Pond is approximately 18.7 acres and has an embankment height of approximately 12 feet from the crest to the toe of the downstream slope. The embankment crest is at elevation 534 the same as the plant site grade and equivalent to the 100-year flood water elevation in the Mississippi River. The interior storage depth of the BGS Main Ash Pond is approximately 8 feet. As stated in the 2020 Annual Inspection, the total volume of impounded CCR and water within the BGS Main Ash Pond at normal water operation elevation is approximately 443,000 cubic yards.

2.3 BGS Economizer Pond

The BGS Economizer Pond is located west of the generating plant and north of the BGS Main Ash Pond. In 1986, BGS constructed the BGS Economizer Pond in the southern and eastern portion of the original footprint of the BGS Upper Ash Pond. The impoundment has resulted from economizer ash that has been deposited since 1986, which created the economizer embankment which is higher than the embankments of the BGS Upper Ash Pond at approximately elevation 548.

Presently, the BGS Economizer Pond receives economizer ash. The economizer ash is sluiced from the generating plant to the east end of the BGS Economizer Pond via a 10-inch diameter polyvinyl chloride pipe at a flow rate of 1.5 cfs (including approximately 10% plant process water). The economizer ash settles out through the water column of the 0.4-acre BGS Economizer Pond while the water flows to the west. The water



discharges from the BGS Economizer Pond through an 18-inch diameter high-density polyethylene pipe into a storm water and process water treatment channel located along the south side of the economizer embankment.

The storm water and process water treatment channel receive runoff from 8 acres surrounding the generating plant. The collected storm water drains into a pump vault located at the toe of the downstream slope of the east embankment of the BGS Economizer Pond. Plant process water flows through an oil/water separator and receives influent flows from the plant floor drains and water treatment process water. After the oil/water separator, the process water discharges into the pump vault. The storm water and process water are then pumped from the vault up to the storm water treatment channel. The storm water treatment channel flows to the west along the south side of the economizer embankment until it discharges through an 18-inch diameter high-density polyethylene pipe located in the southwest corner of the economizer embankment. The water from the storm water treatment channel discharges into a small channel in the southwest corner of the BGS Upper Ash Pond located north of the generating plant entrance road.

The total surface area of the BGS Economizer Pond and economizer embankment is approximately 11 acres and has an embankment height of approximately 13 feet from the crest to the toe of slope on the CCR in the BGS Upper Ash Pond. The interior storage depth of the top of the economizer embankment to the bottom of the original footprint of the BGS Upper Ash Pond is approximately 27 feet. As stated in the 2020 Annual Inspection, the total volume of impounded CCR and water within the BGS Economizer Pond is approximately 478,500 cubic yards.

2.4 BGS Upper Ash Pond

The BGS Upper Ash Pond is located northwest of the generating plant and north of the BGS Main Ash Pond. In 1971, BGS began managing CCR in the BGS Upper Ash Pond.



In 1980, the BGS Main Ash Pond became the primary receiver of CCR and the BGS Upper Ash Pond became a downstream receiver of the BGS Main Ash Pond.

Presently, the BGS Upper Ash Pond receives influent flows from the BGS Main Ash Pond, BGS Economizer Pond, and storm water and process water flow from the generating plant. The influent flows all discharge into a small channel located in the southwest corner of the BGS Upper Ash Pond. The water in the channel routed along the south side of the gravel dike of the BGS Upper Ash Pond until it discharges into the southwest corner of the BGS Upper Ash Pond water body.

The water flows through the BGS Upper Ash Pond water body to the northeast towards a 24-inch wide precast concrete Parshall flume that discharges into a concrete catch basin. The water in the catch basin flows through a 15-inch diameter polyvinyl chloride pipe and discharges into the BGS Lower Pond. Instrumentation associated with the BGS Upper Ash Pond includes a flow meter that monitors the discharges. The discharge from the concrete catch basin enters the Lower Pond. The Lower Pond contains the facility's National Pollutant Discharge Elimination System (NPDES) Outfall 001. The water flows through the NPDES Outfall 001 hydraulic structure, which consists of cast in place weir box.

The total surface area of the BGS Upper Ash Pond is approximately 13.3 acres and has an embankment height of approximately 10 feet from the crest to the toe of the downstream slope. The elevation of the embankments is 531 feet, 3 feet lower than the 100-year flood elevation of the Mississippi River. The embankment is armored with cobble size stone on the crest and both outer and inner embankment slopes to prevent erosion of the embankment during overtopping from extreme flood stage of the Mississippi River. The interior storage depth of the BGS Upper Ash Pond is approximately 7 feet. As stated in the 2020 Annual Inspection, the volume of impounded CCR and water within the BGS Upper Ash Pond is approximately 156,400 cubic yards.



3 HYDROLOGIC AND HYDRAULIC CAPACITY- §257.82(a)

This Report provides hydrologic and hydraulic capacity information for inflow design flood control systems which is intended to:

- 1. Adequately manage flow into each CCR unit during and following the peak discharge inflow of the specified design flood,
- 2. Adequately manage flow from each CCR unit during and following the peak discharge inflow of the specified design flood; and,
- 3. Handle discharge from the CCR unit in accordance with NPDES regulations 40 CFR §257.3-3.

3.1 Hazard Classification and Design Storm

The 2021 Hazard Potential Classification analysis classified both the BGS Ash Seal Pond and BGS Main Ash Pond as significant hazard potential due to the risk that pond contents could enter the BGS Generating Station condenser discharge channel and from there into the Mississippi River. Based on the 2021 Hazard Potential Classification analysis conducted by HHS, both the BGS Economizer Pond and BGS Upper Ash Pond are classified as low hazard potential because if a release occurs, the contents will likely remain on BGS plant property.

The design storm for the BGS Ash Seal Pond and BGS Main Seal Pond is the 1,000-year return event SCS Type II 24-hour storm as designated in §257.82(a)(3)(ii). The design storm for the BGS Economizer Pond and the BGS Upper Ash Pond is the 100-year return event SCS Type II 24-hour storm as designated in §257.82(a)(3)(iii). The total rainfall for the two design events selected from the National Oceanographic and Atmospheric Administration's (NOAA) probabilistic map for the BGS Site coordinates is 10.3 inches for the 1,000-year event and 7.3 inches for the 100-year event, Appendix B.

Since surface water from the BGS Main Ash Pond must route through the BGS Upper Ash Pond, the Inflow Flood will be the 1,000-year event for the complete storm water system.



3.2 Hydrologic and Hydraulic Capacity Methods

3.2.1 BGS Ash Seal Pond

The 1,000-year SCS Type II storm of 10.3 inches accumulates in the storage pool of the BGS Ash Seal Pond without outflow. The total volume of the water is calculated by accumulating the rainfall on the watershed of 7.7 acres and storing the water in the BGS ash seal pond reservoir without discharge. The water elevation in the pond at the end of the design flood event is compared to the crest elevation of the embankment to determine the freeboard remaining at the end of the storm.

3.2.2 Storm Routing Through BGS Economizer Pond, BGS Main Ash Pond, and BGS Upper Ash Pond

The 1,000-year SCS Type II Storm was routed through the CCR units. The flow path is illustrated on Figure 2. The routing was completed using the program Hydraflow by Intelisolve¹. Hydraflow uses the unit hydrograph method to generate a Type II distributed rainfall distribution for each of the drainage area subsets in the watershed. Hydraflow then routes the unit hydrographs through the outlet structures of each BGS pond, storing water within the pond in accordance with the input reservoir capacity of the pond. The proportion of runoff to rainfall for each subunit of the drainage watershed is input based on the characteristics of the area.

The subareas of the flood control plan are shown in Figure 1 and include:

- Storm water runoff from 8 acres of the plant site that is collected at the base of the BGS Economizer Pond at elevation 534 feet and pumped to the top of the BGS Economizer Pond at elevation 548 feet. Due to pump capacity, the flow is limited to a peak of 17.8 cubic feet per second (cfs).
- 2. Storm water runoff from 17 acres of the BGS Main Ash Pond.
- 3. Storm water runoff from 25 acres of the BGS Economizer Pond and BGS Upper Ash Pond combined.



¹Intelisolve. Pond Routing Software Hydraflow, 2002

4. In addition to the storm water flows identified above, a BGS process water flow of 1.0 cfs is added to the BGS Main Ash Pond and 1.5 cfs is added to the plant site water flow discharged to the BGS Economizer Pond, Appendix A.

3.3 Hydrologic and Hydraulic Capacity Input and Assumptions

This section identifies the input and assumptions for the hydrologic and hydraulic capacity calculations. The input for each drainage subarea is:

Sub-Area	Acreage	Curve Number (CN)	Slope (%)	Hydraulic Length (ft)
Plant Site	8	90	1.0	600
Main Ash Pond	17	85	1.5	1,250
Economizer Ditch	Open channel	Mannings n = 0.009	0	1,000
Economizer and Upper Ash Ponds	25	87	1.5	1,250

The CN for the plant site is typical of industrial sites with considerable paved areas. The CN for the ash pond areas reflects the ability to infiltrate into unsaturated CCR which is exposed and available to store infiltrated water early in the storm distribution.

Three reservoirs are part of the flood routing system and consist of the BGS Main Ash Pond, the BGS Economizer Ditch and Pond, and the BGS Upper Ash Pond. The outlet structures for each pond and the maximum storage capacity of the pond at maximum flow are:

Pond Reservoir	Total Storage (acre-feet)	Outlet Structure	Invert Elevation (feet)	Secondary Outlet Structure	Invert Elevation (feet)
BGS Main Ash Pond	11.0	2 – 15-inch CMP culverts	531.1	none	
BGS Economizer Pond	0.21	1 – 18-inch HDPE	544.0	1 – 12-foot-wide rock filled weir	546.5
BGS Upper Ash Pond	51.8	Monitoring Flume + 15- inch PVC pipe	527.4	1 – 14-inch pipe with manual operated gate valve	529.0

The details of the BGS Economizer Pond and BGS Upper Ash Pond outfall structures are



shown in Appendix C. There are no available design drawings for the BGS Main Ash Pond outfall which is shown in the pictures contained in Appendix C.

The assumptions which impact the analysis of the flood routing are:

- The outfall structures are not submerged on the tailwater and are therefore inlet restricted. This is a realistic assumption for the BGS Main Ash Pond and the BGS Economizer Pond. For the BGS Upper Ash Pond with the outlet invert of 523.4 feet, the tail could be submerged if the Mississippi River is in flood stage above approximately 525 feet. This is a reasonable assumption since Mississippi River flood stage and design flood are unlikely to be coincident.
- 2. The emergency overflow pipe on the BGS Upper Ash Pond has a manually operated valve. It is assumed that this valve is closed, and all the flood flow must past through the flume and 15-inch diameter discharge pipe.
- 3. The rainfall runoff on the Plant Area (Figure 2) is stored on the plant area when the pump capacity of the lift pumps is exceeded, and the volume is recovered as the pumps catch-up after the precipitation event. This volume could overflow directly to the BGS Upper Ash Pond reducing the duration of the maximum pumping capacity event.
- 4. To be conservative, no exfiltration of water through the bottom of the ponds is considered in the analysis. Since the natural soil under the ponds is clay the assumption likely holds for all ponds except for the BGS Economizer Pond where a substantial unsaturated zone is available to accept exfiltrate water during the rainfall event.
- 5. The beginning water elevation condition in the ponds at the start of the inflow flood is the normal operating elevations, or in the case of the BGS Ash Seal Pond, no water.



4 INFLOW DESIGN FLOOD CONTROL SYSTEM PLAN

The inflow flood caused by a 1,000-year return event SCS Type II storm distribution is routed through the CCR pond system at BGS. The routing includes the BGS Ash Seal Pond that operates as a no discharge pond and the remaining ash ponds that discharge through NPDES Outfall 001.

4.1 BGS Ash Seal Pond Plan

The 7.7 acres of the BGS Ash Seal Pond will accumulate 6.6 acre-feet of water during the design storm. Without discharging water, the final water elevation in the pond will be 533.4 feet with a freeboard of 0.6 feet. The results of the calculation of capacity are presented in Appendix D.

The area of the BGS Ash Seal pond is located on the natural levee deposits of the Mississippi River and unlike the other ash ponds located west of the levee, Figure 1, has a sandy natural soil under the pond. The actual water elevation in the pond is likely to be lower due to exfiltration of water during the storm event.

4.2 BGS Ash Ponds Draining to BGS Upper Ash Pond Outfall

The 50 acres of general plant area, BGS Main Ash Pond, BGS Economizer Pond, and BGS Upper Ash Pond will discharge a maximum flow of 9.4 cfs at the BGS Upper Ash Pond outfall during the inflow flood routing. Between the BGS Main Ash Pond and the BGS Upper Ash Pond, 36-acre feet of water will be stored above the normal operating elevation of the two ponds at maximum discharge. The storage will occur with a water elevation of 533.4 feet in the BGS Main Ash Pond (freeboard 0.6 foot) and 530.3 feet in the BGS Upper Ash Pond (freeboard 0.7 feet). The hydrographs, reservoir and outlet details are presented in Appendix E.

The flow from the BGS Economizer Pond includes the storm water from the plant area and the process water flow of 1.5 cfs. Based on both 4,000 gpm pumps in the combined storm water and process water sump operating, the maximum flow to the storm water



and process water treatment channel on top of the BGS Economizer Pond is 19.3 cfs. This flow will occur for approximately 1 hour during the peak of the inflow flood event and will cause the capacity of the existing 18-inch HDPE outfall pipe from the BGS Economizer Pond to be exceeded. To route the entirety of the peak inflow flood event the previously existing 12-inch diameter steel pipe was removed and replaced with a 12foot wide 18-inch deep rock filled weir. The rock filled weir, constructed in June 2017, will route the excess flow through the riprap and along the downstream slope of the embankment into the BGS Upper Ash Pond.

Approximately 0.22-acre feet of water will be stored above the normal operating elevation of the BGS Economizer Pond at maximum discharge. The storage will occur with a water elevation of 547.5 feet (freeboard 0.5 feet). The hydrographs, reservoir and outlet details are presented in Appendix E.

07/16/2021 - Classification: Internal - ECRM12625227



5 QUALIFIED PROFESSIONAL ENGINEER CERTIFICATION

To meet the requirements of 40 CFR 257.82(c)(5), I Mark W. Loerop hereby certify that I am a licensed professional engineer in the State of Iowa; and that, to the best of my knowledge, all information contained in this document is correct and the document was prepared in compliance with all applicable requirements in 40 CFR 257.82.

This Inflow Flood Control Plan was prepared and certified by a qualified professional engineer. The next periodic review of this Inflow Flood Control Plan is required to be complete by April 12, 2026.



Name:

Date: June 2021

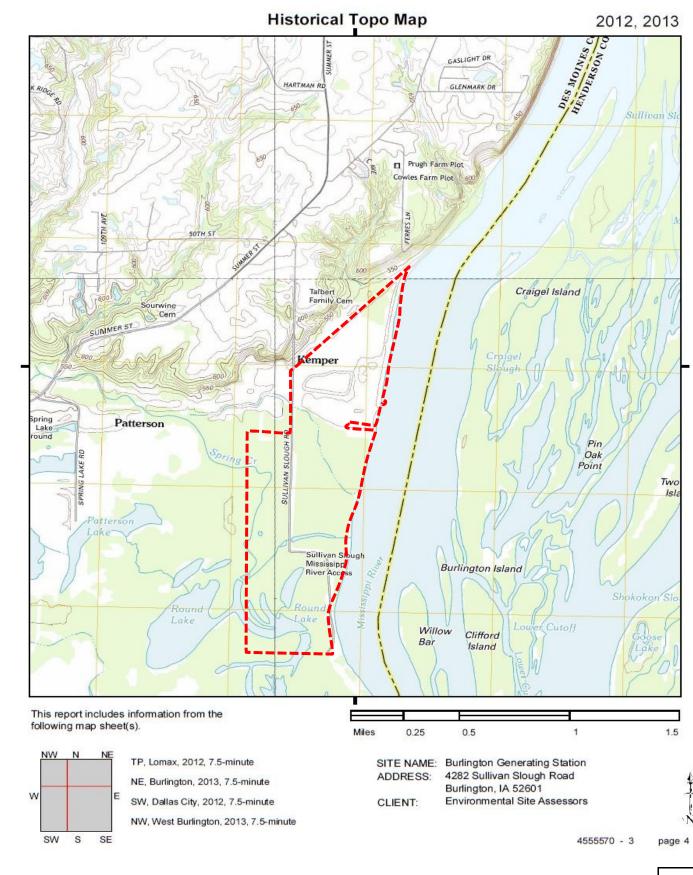


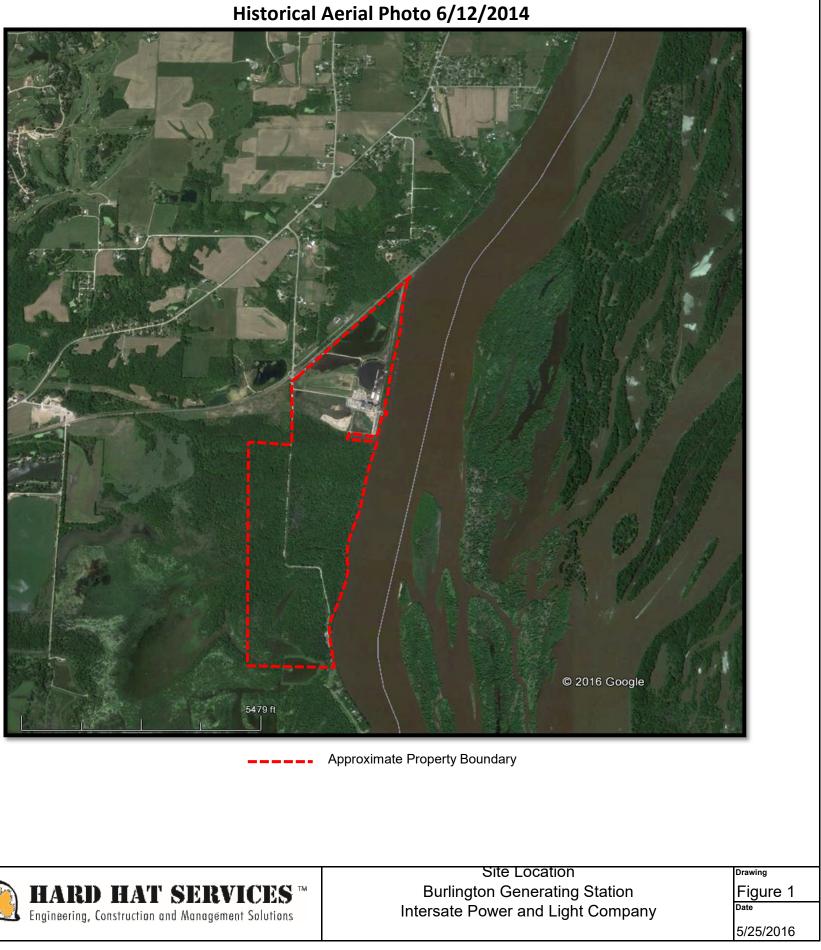
FIGURES

Alliant Energy Interstate Power and Light Company Burlington Generating Station Burlington, Iowa

Inflow Design Flood Control System Plan

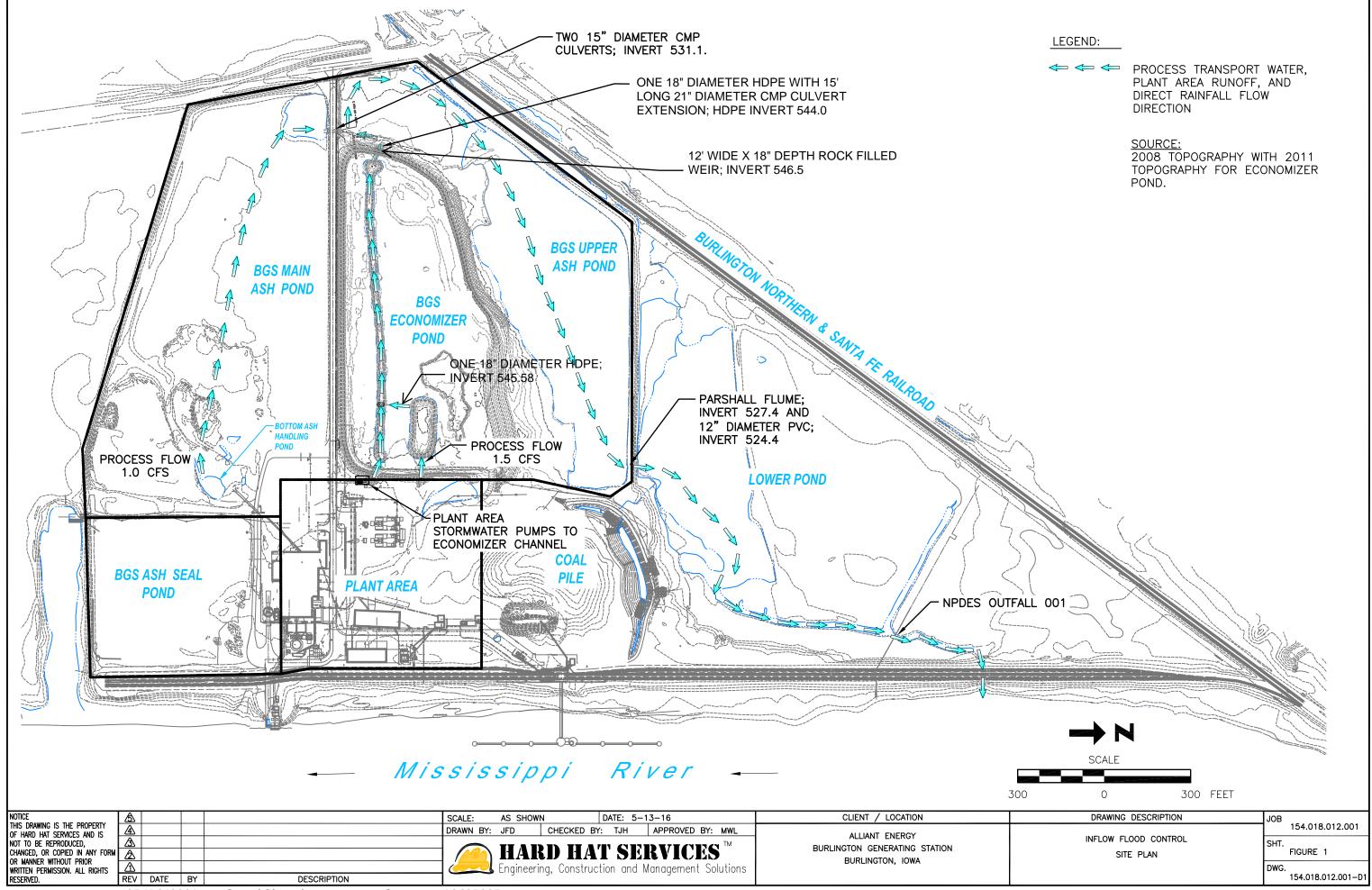










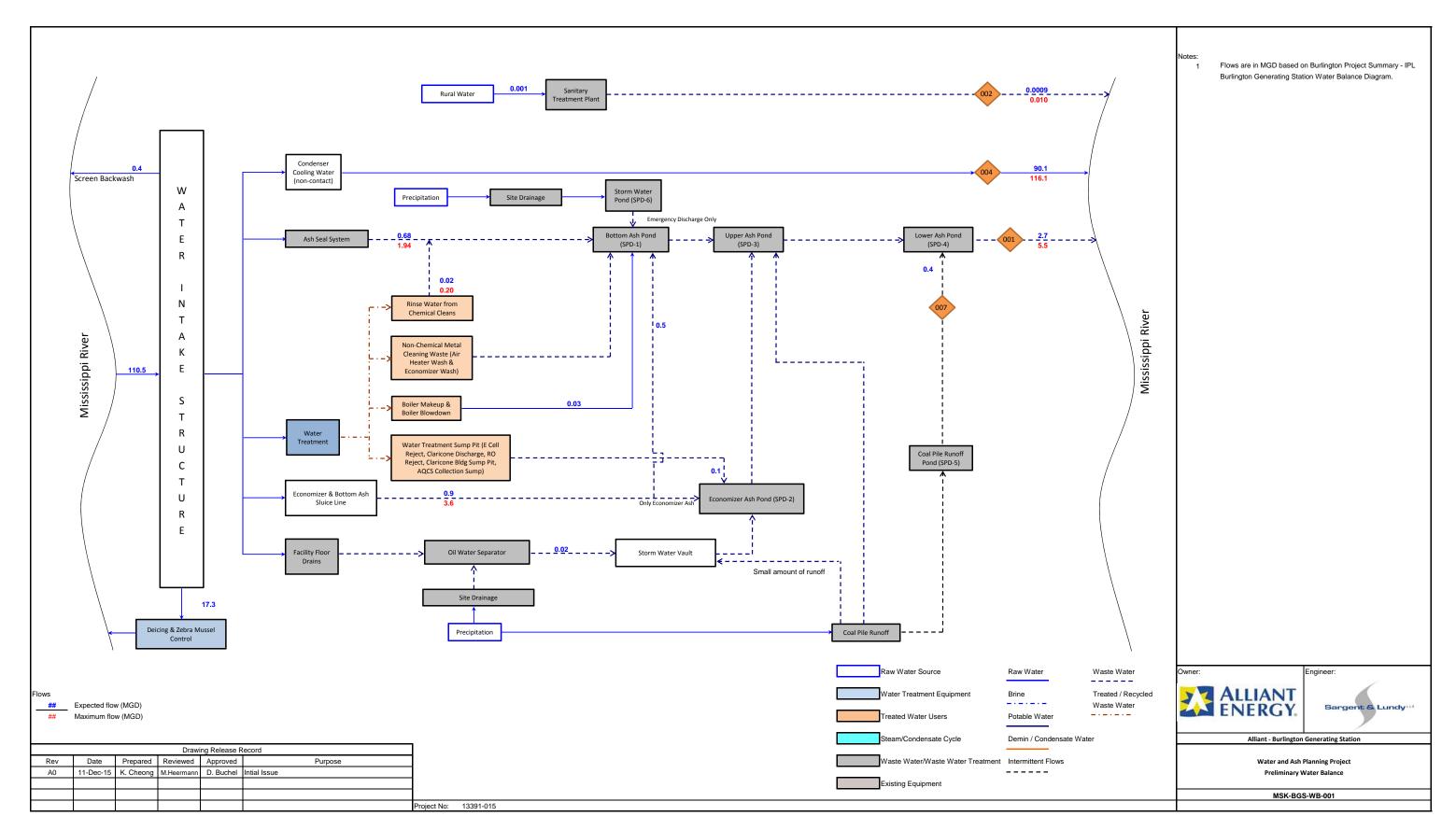


APPENDIX A – BGS Water Balance Chart

Alliant Energy Interstate Power and Light Company Burlington Generating Station Burlington, Iowa

Inflow Design Flood Control System Plan





APPENDIX B – NOAA Storm Frequency

Alliant Energy Interstate Power and Light Company Burlington Generating Station Burlington, Iowa

Inflow Design Flood Control System Plan





NOAA Atlas 14, Volume 8, Version 2 Location name: Burlington, Iowa, US* Latitude: 40.7426°, Longitude: -91.1201° Elevation: 545 ft* * source: Google Maps



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹												
Duration	Average recurrence interval (years)											
Duration	1	2	5	10	25	50	100	200	500	1000		
5-min	0.384 (0.326-0.458)	0.449 (0.381–0.537)	0.555 (0.470-0.664)	0.642 (0.541-0.770)	0.760 (0.620-0.924)	0.849 (0.680-1.04)	0.937 (0.728–1.16)	1.03 (0.769–1.29)	1.14 (0.826–1.46)	1.23 (0.869–1.59)		
10-min	0.562 (0.477-0.671)	0.658 (0.559–0.786)	0.813 (0.688-0.973)	0.940 (0.792–1.13)	1.11 (0.908–1.35)	1.24 (0.995–1.52)	1.37 (1.07–1.71)	1.50 (1.13–1.89)	1.67 (1.21–2.14)	1.79 (1.27-2.32)		
15-min	0.685 (0.582–0.818)	0.802 (0.681–0.958)	0.991 (0.840-1.19)	1.15 (0.966–1.38)	1.36 (1.11–1.65)	1.52 (1.21–1.86)	1.67 (1.30-2.08)	1.83 (1.37–2.31)	2.04 (1.48–2.61)	2.19 (1.55-2.83)		
30-min	0.952 (0.810-1.14)	1.12 (0.953–1.34)	1.39 (1.18–1.67)	1.62 (1.36–1.94)	1.91 (1.56–2.32)	2.14 (1.71–2.62)	2.36 (1.83-2.92)	2.57 (1.93-3.24)	2.85 (2.07-3.65)	3.06 (2.17-3.96)		
60-min	1.23 (1.05–1.47)	1.43 (1.22–1.71)	1.77 (1.50-2.12)	2.05 (1.73-2.46)	2.44 (2.00-2.98)	2.75 (2.20-3.38)	3.06 (2.38-3.81)	3.38 (2.54-4.27)	3.81 (2.77-4.89)	4.14 (2.94–5.36)		
2-hr	1.51 (1.30–1.79)	1.75 (1.49–2.07)	2.14 (1.83–2.54)	2.48 (2.11–2.95)	2.97 (2.46-3.61)	3.36 (2.72-4.11)	3.76 (2.96-4.66)	4.19 (3.18–5.26)	4.77 (3.50-6.09)	5.22 (3.74–6.71)		
3-hr	1.69 (1.45–1.99)	1.93 (1.66–2.28)	2.36 (2.02–2.78)	2.74 (2.33–3.23)	3.29 (2.75-4.00)	3.75 (3.06-4.58)	4.24 (3.35-5.24)	4.75 (3.63–5.97)	5.48 (4.04-6.98)	6.06 (4.36-7.74)		
6-hr	1.99 (1.72–2.32)	2.28 (1.98–2.66)	2.80 (2.42-3.28)	3.27 (2.81–3.83)	3.97 (3.34–4.79)	4.55 (3.75-5.52)	5.18 (4.13-6.36)	5.85 (4.51-7.29)	6.79 (5.06-8.59)	7.55 (5.48-9.58)		
12-hr	2.29 (2.00-2.65)	2.68 (2.34–3.10)	3.35 (2.92–3.89)	3.95 (3.42-4.58)	4.81 (4.07–5.75)	5.52 (4.57-6.63)	6.27 (5.04-7.62)	7.06 (5.47-8.71)	8.16 (6.12-10.2)	9.03 (6.60-11.4)		
24-hr	2.66 (2.34-3.04)	3.11 (2.74–3.57)	3.90 (3.42-4.48)	4.58 (4.00-5.27)	5.57 (4.75-6.58)	6.37 (5.31-7.57)	7.21 (5.83-8.68)	8.09 (6.32-9.88)	9.30 (7.03-11.5)	10.3 (7.57–12.8)		
2-day	3.11 (2.76–3.53)	3.58 (3.17–4.06)	4.38 (3.87–4.98)	5.08 (4.47-5.80)	6.11 (5.25-7.16)	6.95 (5.85-8.19)	7.83 (6.40-9.35)	8.76 (6.91–10.6)	10.1 (7.67–12.4)	11.1 (8.24–13.7)		
3-day	3.38	3.89	4.74	5.48	6.54	7.39	8.27	9.20	10.5	11.5		

07/16/2021 - Classification: Internal - ECRM12625227

http://hdsc.nws.noaa.gov/hdsc/pfds/pfds_printpage.html?lat=40.7426&lon=-91.1201&data=depth&units=english&series=pds 4/1/2016

	(3.01-3.82)	(3.46-4.39)	(4.20-5.36)	(4.83–6.21)	(5.63-7.59)	(6.24-8.64)	(6.78-9.81)	(7.28–11.1)	(8.02–12.8)	(8.58–14.1)
4-day	3.61 (3.23-4.06)	4.15 (3.70-4.67)	5.04 (4.49–5.69)	5.81 (5.15-6.57)	6.90 (5.96-7.97)	7.77 (6.57–9.03)	8.66 (7.12–10.2)	9.58 (7.60–11.5)	10.8 (8.32–13.2)	11.8 (8.87–14.5)
7-day	4.26 (3.83-4.75)	4.84 (4.35–5.41)	5.81 (5.20-6.50)	6.63 (5.91–7.44)	7.79 (6.76-8.91)	8.70 (7.40–10.0)	9.63 (7.96-11.3)	10.6 (8.45–12.6)	11.9 (9.18–14.4)	12.9 (9.73–15.7)
10-day	4.86 (4.39–5.40)	5.48 (4.95-6.09)	6.52 (5.86-7.25)	7.39 (6.61–8.25)	8.61 (7.51–9.81)	9.57 (8.18–11.0)	10.6 (8.76–12.3)	11.6 (9.27–13.7)	12.9 (10.0–15.5)	14.0 (10.6–17.0)
20-day	6.63 (6.03-7.29)	7.42 (6.75-8.17)	8.71 (7.90–9.61)	9.79 (8.83–10.8)	11.3 (9.88–12.7)	12.4 (10.7–14.1)	13.5 (11.3–15.6)	14.7 (11.9–17.2)	16.2 (12.7–19.3)	17.4 (13.3–20.9)
30-day	8.11 (7.42-8.88)	9.10 (8.31–9.96)	10.7 (9.73–11.7)	12.0 (10.9–13.2)	13.7 (12.1–15.3)	15.0 (13.0–16.9)	16.3 (13.7–18.7)	17.6 (14.3–20.5)	19.3 (15.1–22.8)	20.5 (15.8–24.6)
45-day	10.0 (9.19–10.9)	11.3 (10.3–12.3)	13.3 (12.2–14.5)	14.9 (13.6–16.3)	17.0 (15.0–18.8)	18.6 (16.1–20.8)	20.1 (16.9–22.8)	21.6 (17.6–24.9)	23.4 (18.5–27.5)	24.7 (19.1–29.5)
60-day	11.6 (10.7–12.6)	13.2 (12.1–14.3)	15.6 (14.3–16.9)	17.5 (16.0–19.1)	20.0 (17.7–22.0)	21.8 (18.9–24.2)	23.5 (19.8–26.5)	25.1 (20.5–28.8)	27.1 (21.4–31.7)	28.5 (22.1–33.8)

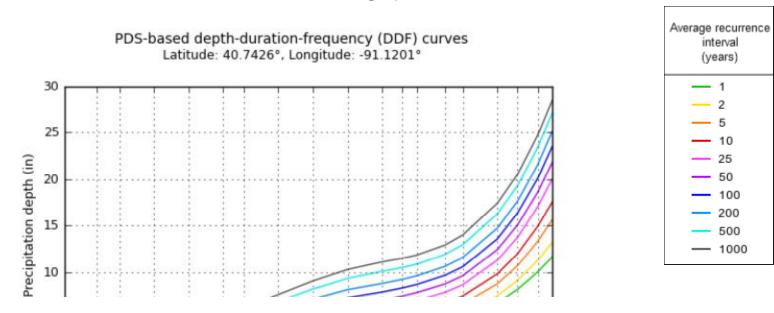
Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

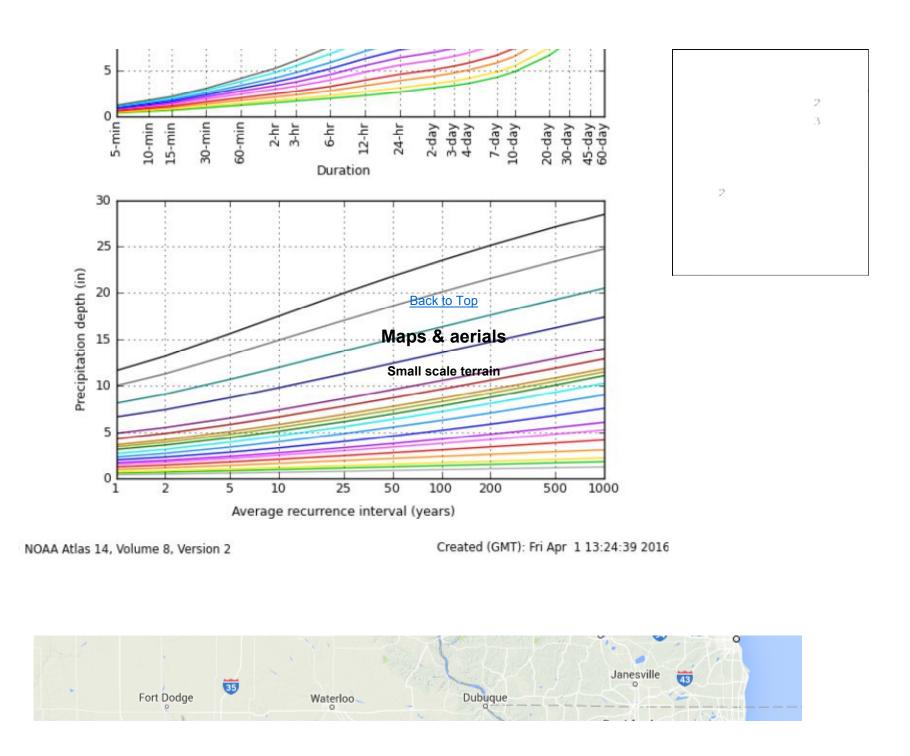
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PF graphical

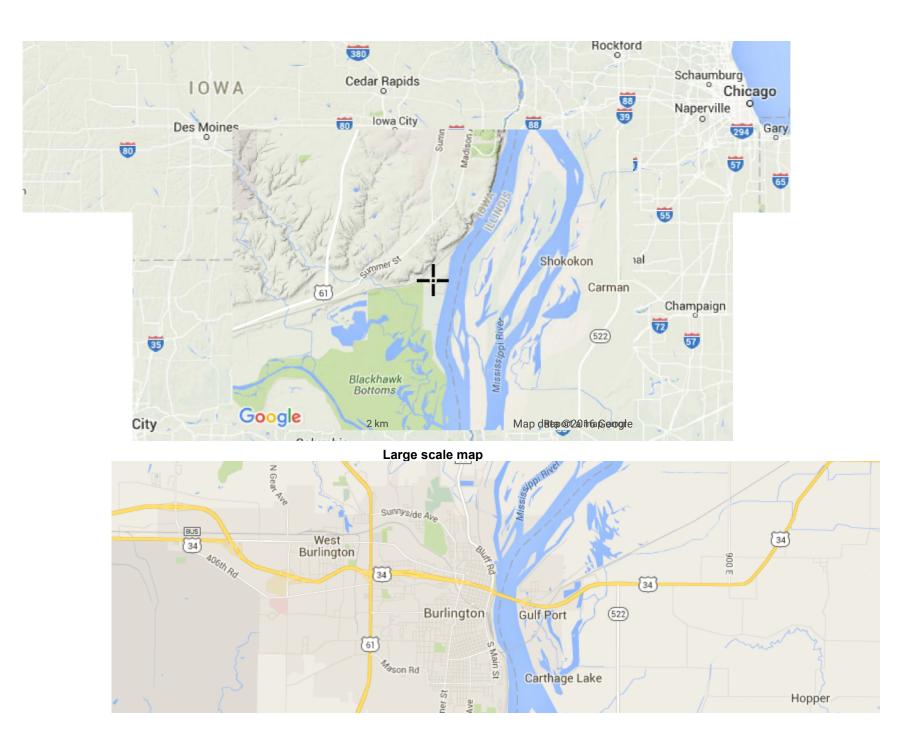


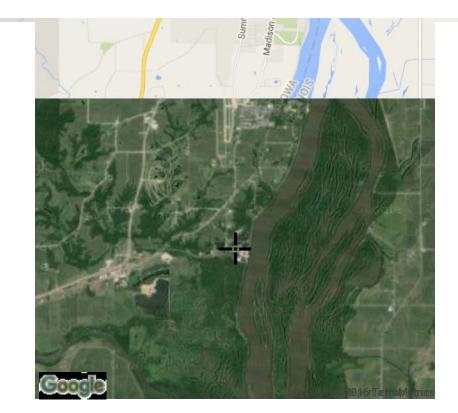
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US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: HDSC.Questions@noaa.gov

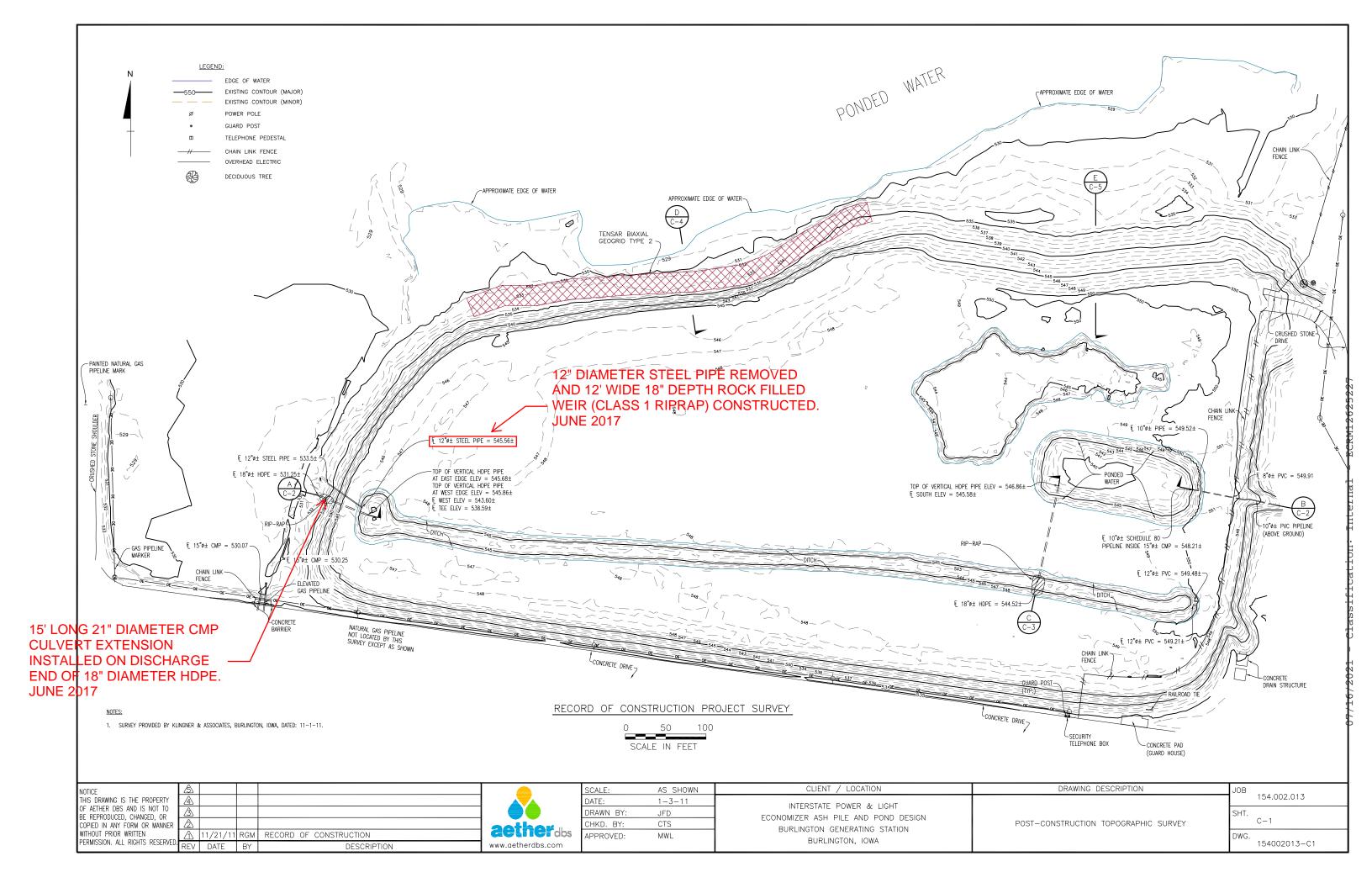
Disclaimer

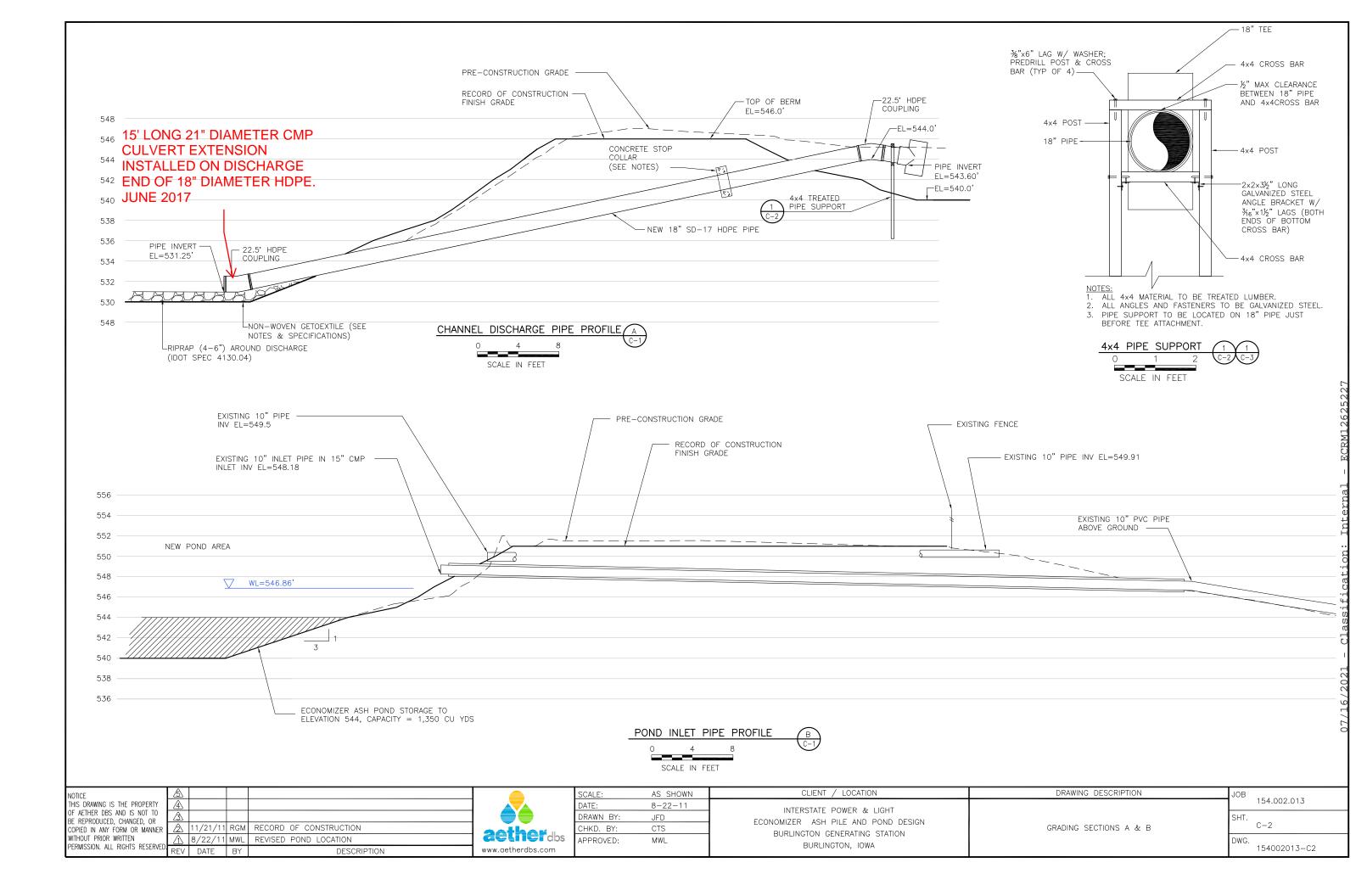
APPENDIX C – Outfall Drawings

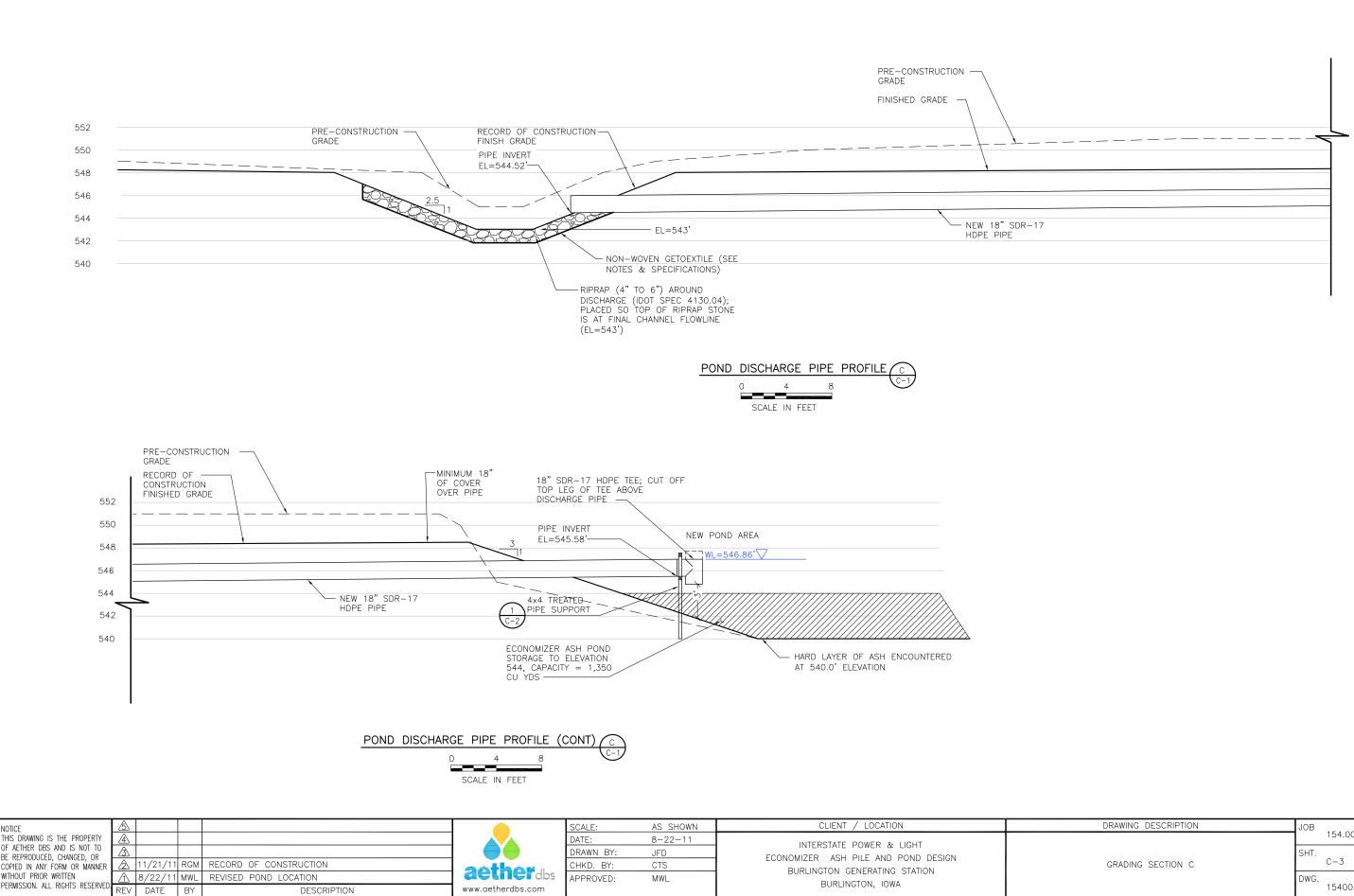
Alliant Energy Interstate Power and Light Company Burlington Generating Station Burlington, Iowa

Inflow Design Flood Control System Plan

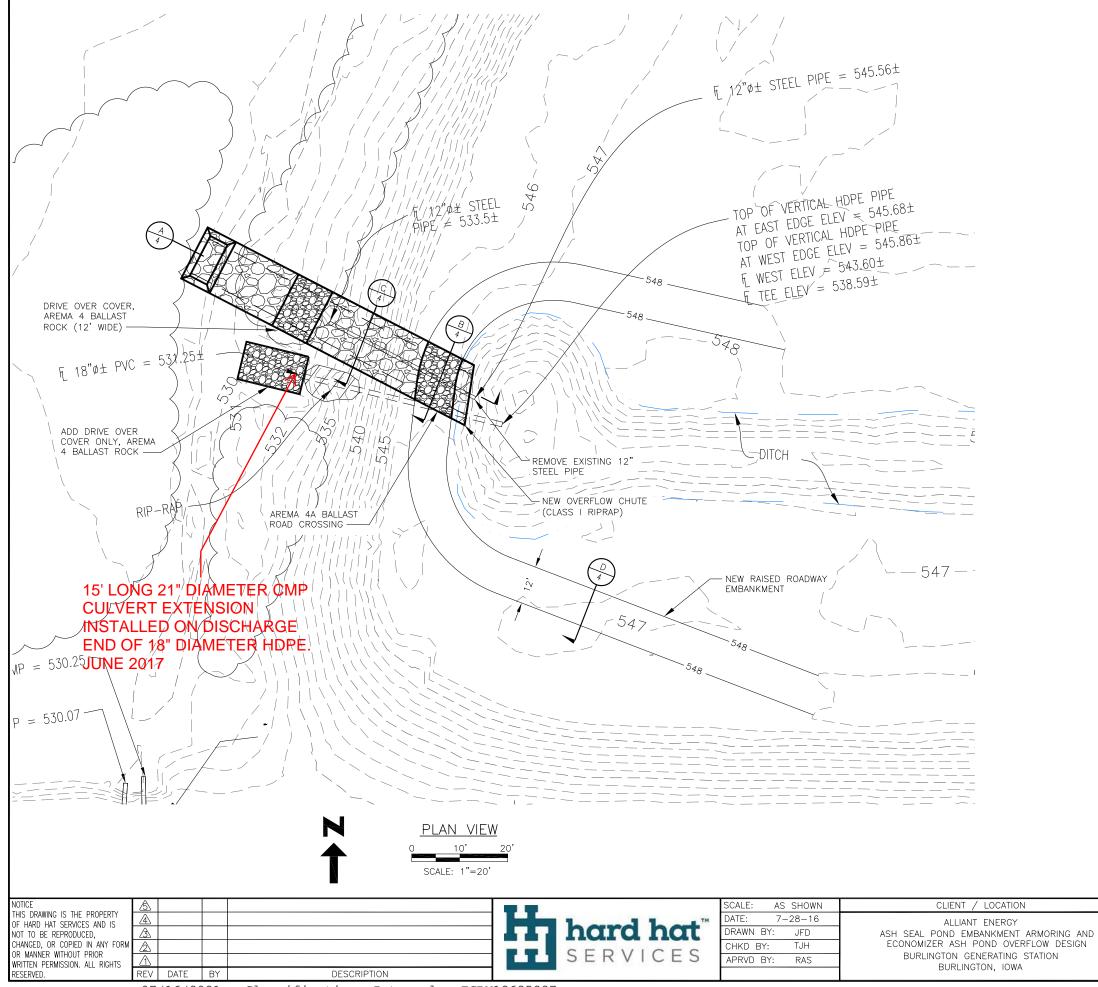








DRAWING DESCRIPTION	JOB
	154.002.013
GRADING SECTION C	SHT. C-3
	DWG. 154002013-C3

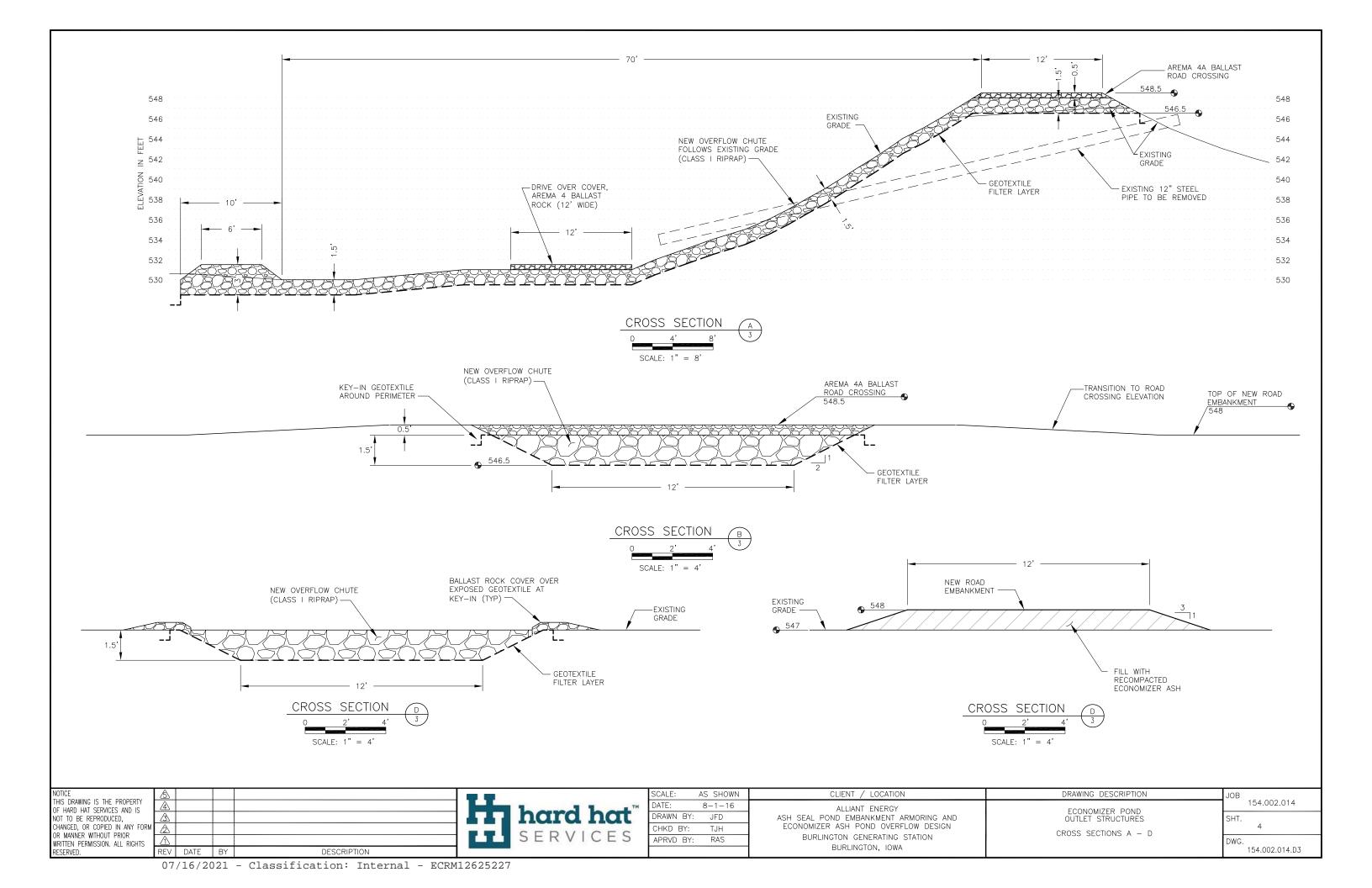


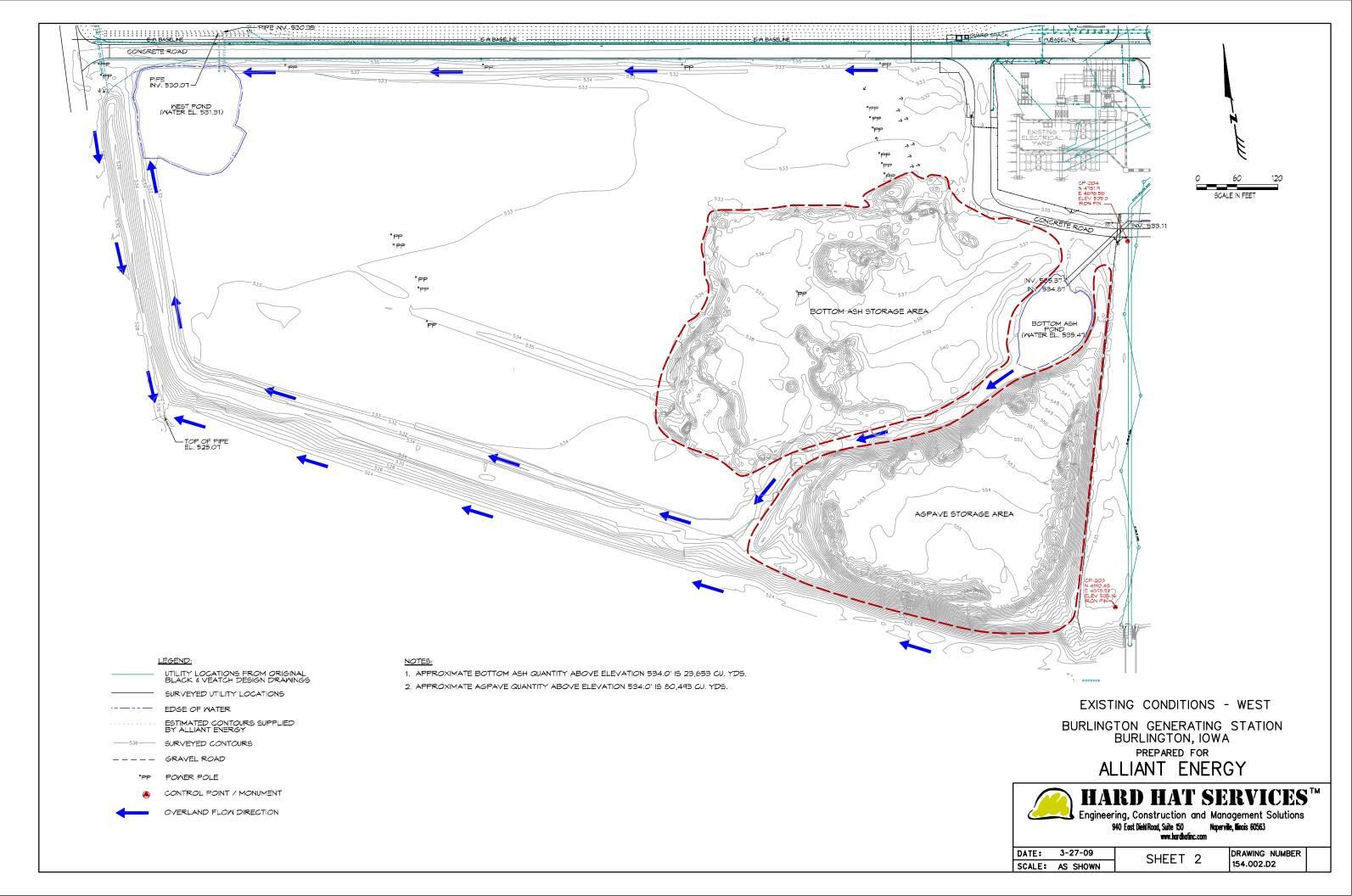
^{07/16/2021 -} Classification: Internal - ECRM12625227

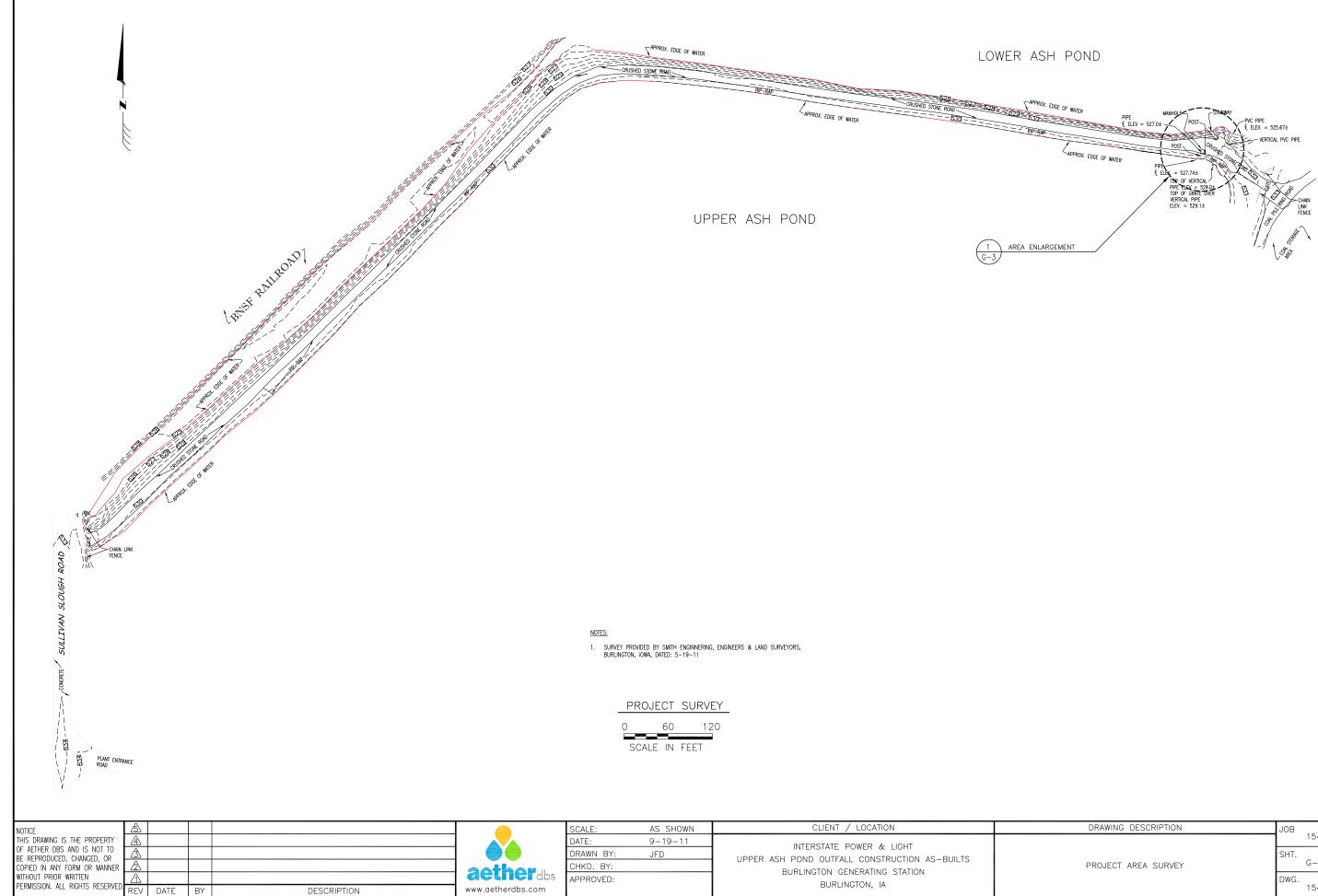
SOURCE: 2008 TOPOGRAPHY WITH 2011 TOPOGRAPHY FOR ECONOMIZER POND, KLINGER & ASSOCIATES.

- INSTRUCTIONS: 1. RETAIN EXISTING HDPE NORMAL FLOW DISCHARGE PIPE. 2. REMOVE EXISTING FLOOD FLOW OVERFLOW PIPE AND
- REMOVE EXISTING FLOOD FLOW OVERFLOW PIPE AND REPLACE WITH ROCK FILLED OVERFLOW CHUTE.
 PROVIDE SURFACE OF BALLAST ROCK ON BOTH CREST AND TOE OF NEW ROCK CHUTE AND ON TOE OF EXISTING RIPRAP OF HDPE DISCHARGE PIPE.
 RAISE CREST OF EMBANKMENT TO ELEVATION 548 AT
- DISCHARGE LOCATION.

DRAWING DESCRIPTION	JOB
ECONOMIZER POND	154.002.014
OUTLET STRUCTURES	SHT.
NEW OVERFLOW CHUTE	3
	DWG.
	154.002.014.D3

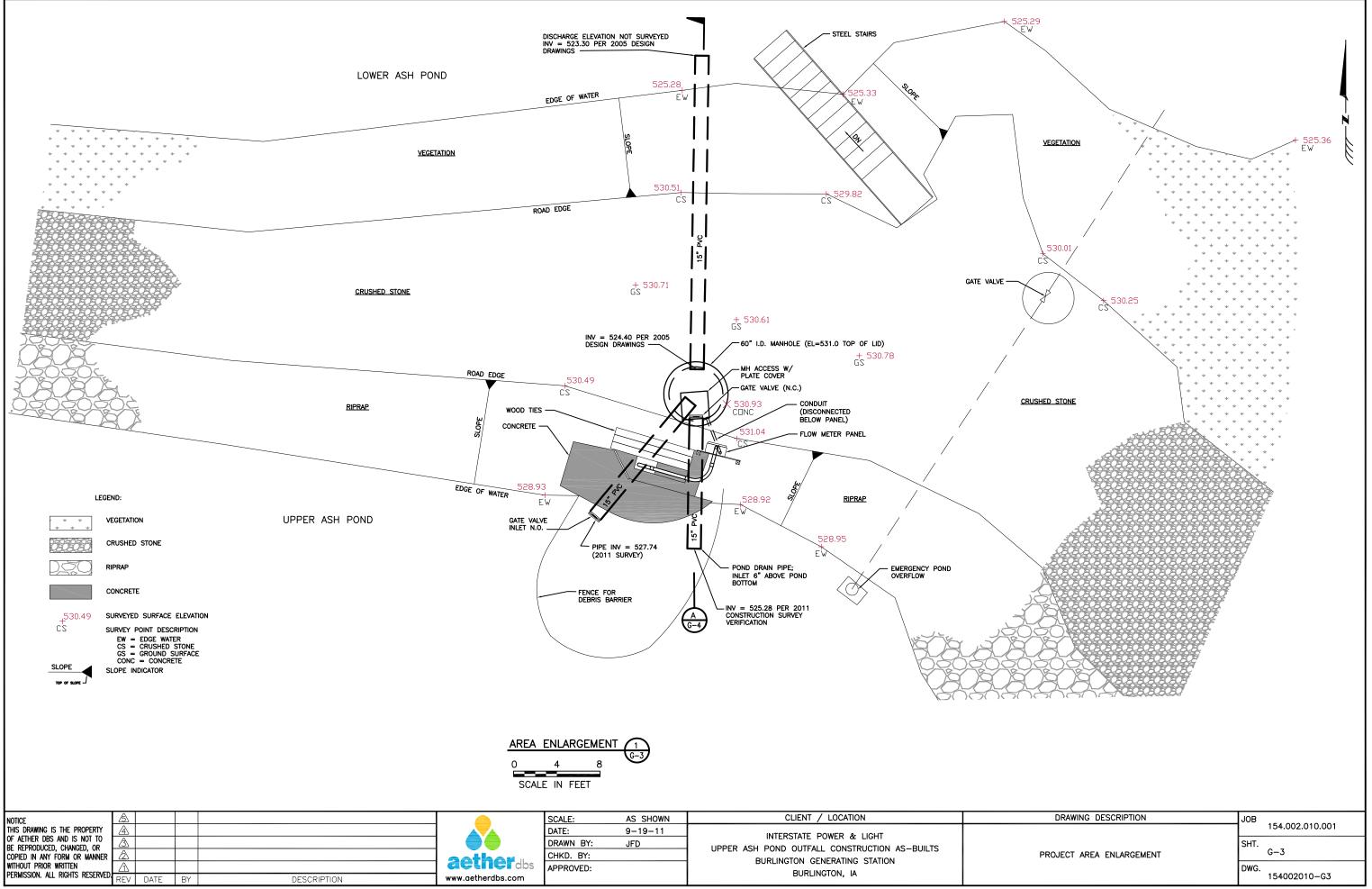






07/16/2021 - Classification: Internal - ECRM12625227

DRAWING DESCRIPTION	JOB
	154.002.010.001
PROJECT AREA SURVEY	SHT. G-2
	DWG. 154002010-G2



APPENDIX D – BGS Ash Seal Pond Analysis

Alliant Energy Interstate Power and Light Company Burlington Generating Station Burlington, Iowa

Inflow Design Flood Control System Plan

Interstate Power and Light Company – Burlington Generating Station Inflow Design Flood Control System Plan



HARD HAT SERVICES Construction and Management Sc SHEET NO. OF PROJECT NO. DATE APRIL 24. 2016 ASH SEAL POND BY TOT CKD MWL FLOOD ANALYSIS NELOW 2 3 PONDED AREA = 5.7 ACRES 4 ADDITIONAL WATERSHED = 2.0 ACRES 5 6 TOTAL = 7.7 ALRES 7 8 9 1000 PEAR RETURN STORM = 10,3 INCHES 10 TOTAL STORED VOLLIME (NO EXCLUTRATION) 11 12 VOLUME = 10.3 INCHES / 12 IN/CH × T. TACRES 13 14 = 6.6 ACRE/FEET = 237,500 FRET 3 15 16 17 18 19 BOTTOM ELEVATION IN SOUTH WEST CORNER 531 FT. 20 21 AREA 2 531 30,000 D' > 1.4 ACRE-FT -AREA 2 532 90,000 D' > 3.3 ACRE-FT -AREA 2 533 200,000 D' > 3.3 ACRE-FT -AREA 2 534 200,000 D' > 4.6 ACRE-FT -22 23 24 25 9.3 ACREFT 26 27 WATER ELEVATION = 6.6 MAR. FT - 1.4 MARCH - 3.3 ACLE FT 28 4. GACRE-FT 29 = 533.4 FT ~ 30 31 32 33 34 * AREA SOUTH OF MATH PLANT (FLYASH HAN DLONG AREA) 35 ** ELEVATIONS FROM GOOGLE EARTH 36

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APPENDIX E – Ash Pond Hydraulic Analysis

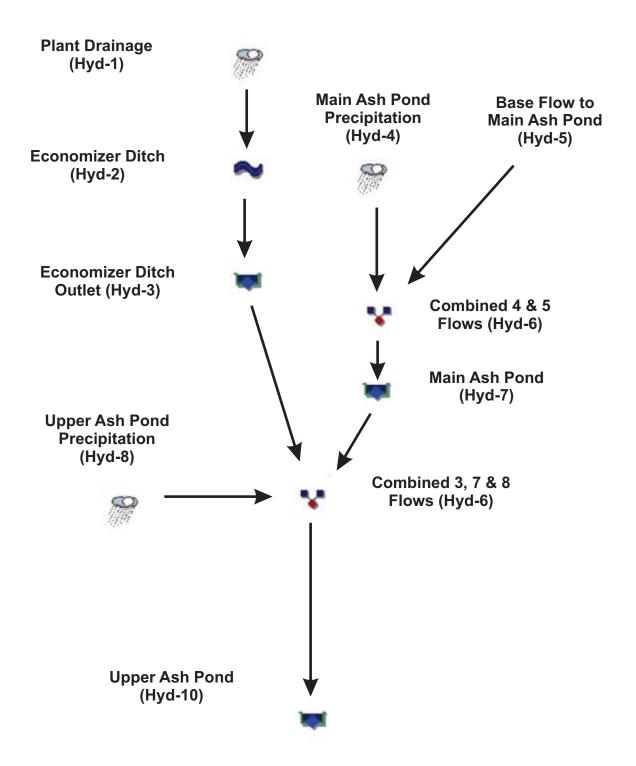
Alliant Energy Interstate Power and Light Company Burlington Generating Station Burlington, Iowa

Inflow Design Flood Control System Plan

Interstate Power and Light Company – Burlington Generating Station Inflow Design Flood Control System Plan



HYDRAFLOW MODEL DIAGRAM



Hydrograph Summary Report

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Maximum storage (acft)	Hydrograph description
1	Manual	19.30	4	704	8.743				Plant Area Drainage Pumps
2	Reach	19.30	4	836	8.780	1			Economizer Ditch
3	Reservoir	19.30	4	836	8.698	2	547.51	0.221	Econ. Ditch Outlet
4	SCS Runoff	136.99	4	728	12.342				Main Ash Pond Runoff
5	Manual	1.00	4	0	2.011				Main Base Flow
6	Combine	137.99	4	728	14.353	4, 5			Main Ash Pond Inflow
7	Reservoir	13.71	4	784	14.348	6	533.37	6.175	Main Ash Pond Outflow
8	SCS Runoff	237.81	4	724	18.127				Economizer Area Runoff
9	Combine	264.17	4	724	41.173	3, 7, 8			Upper Ash Pond Inflow
10	Reservoir	9.35	4	1460	41.023	9	530.23	29.621	Upper Ash Pond Outflow
Proj.	. file: Burlin	naton-14	L.apw	F	Leturn Pe	⊥ riod: 1,00	0 vr	Run date	e: 07-18-2017

Hydraflow Hydrographs by Intelisolve

Hydraflow Hydrographs by Intelisolve

Hyd. No. 1

Plant Area Drainage Pumps

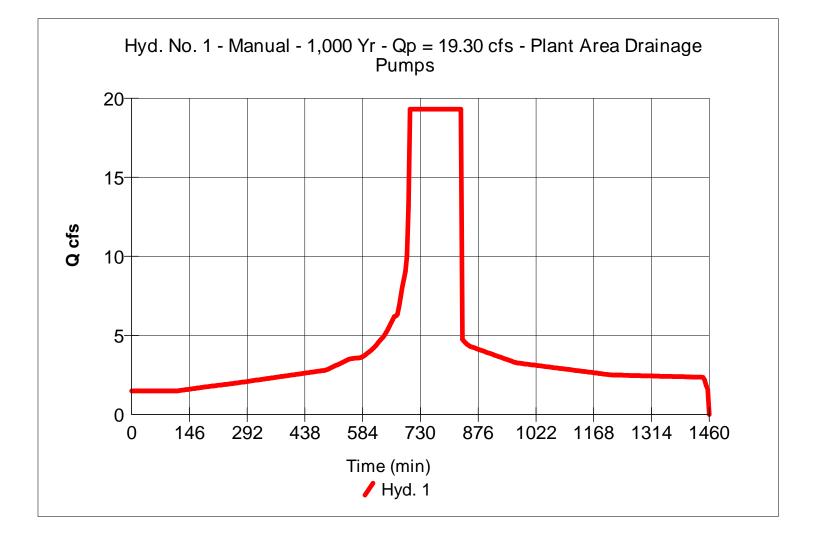
Hydrograph type	= Manual	Peak discharge	= 19.30 cfs
Storm frequency	= 1000 yrs	Time interval	= 4 min

Hydrograph Discharge Table

Time -	- Outflow	Time	Outflow
(min	cfs)	(min	cfs)
(min 248 268 288 308 328 348 368 388 408 428 448 468 488 508 528 548 568 548 568 548 608 628 648 668 688 708 728 748 768 788	cfs) 1.94 2.00 2.07 2.15 2.22 2.29 2.36 2.44 2.51 2.58 2.65 2.72 2.80 3.00 3.24 3.49 3.57 3.72 4.12 4.68 5.41 6.22 8.55 19.30 << 19.30 << 19.30 << 19.30 << 19.30 << 19.30 << 19.30 <<		
808 828 848 868 888	19.30 << 19.30 << 4.44 4.20 4.02	End	
908	3.84		

Peak discharge	= 19.30 cfs
Time interval	= 4 min

Hydrograph Volume = 8.743 acft



Hyd. No. 2

Economizer Ditch

Hydrograph type=ReachStorm frequency=1000 yrsInflow hyd. No.=1Reach length=1000.0 ftManning's n=0.009Side slope=2.5:1Rating curve x=0.253Ave. velocity=0.78 ft/s	Peak discharge Time interval Section type Channel slope Bottom width Max. depth Rating curve m Routing coeff.	 = 19.30 cfs = 4 min = Trapezoidal = 0.00 % = 5.00 ft = 4.00 ft = 1.350 = 0.2240
--	--	--

Modified Att-Kin routing method used.

Hydrograph Discharge Table

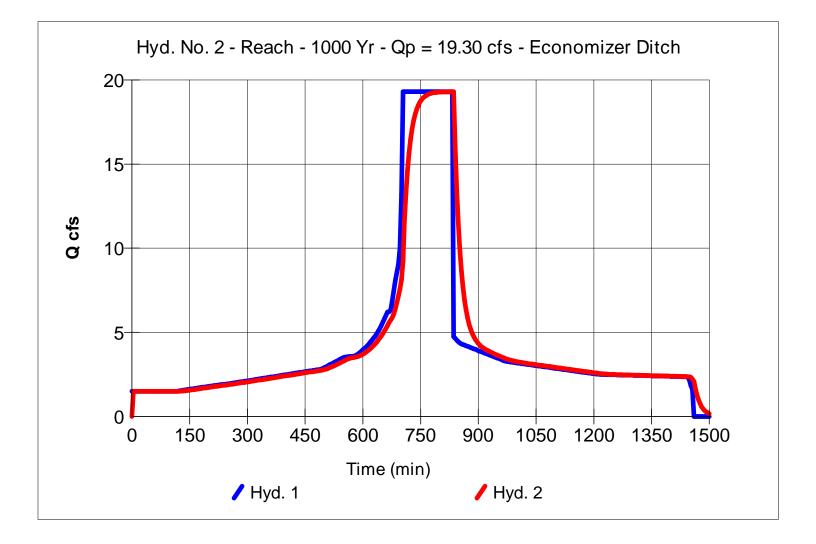
Time	Inflow	Outflow
(min)	cfs	cfs
280	2.05	1.98
300	2.12	2.05
320	2.19	2.13
340	2.26	2.20
360	2.34	2.27
380	2.41	2.34
400	2.48	2.42
420	2.55	2.49
440	2.62	2.56
440 460 480	2.70 2.77	2.63 2.70
500	2.91	2.78
520	3.14	2.96
540	3.39	3.18
560	3.55	3.41
580	3.61	3.52
600	3.96	3.70
620	4.44	4.04
640	5.06	4.54
660	6.00	5.24
680	7.46	6.08
700	13.20	8.13
720	19.30 <<	15.66
740	19.30 <<	18.28
760	19.30 <<	19.01
780	19.30 <<	19.22
800	19.30 <<	19.28
820	19.30 <<	19.29
840	4.64	16.04
860	4.27	7.69
880	4.09	5.17
900	3.91	4.33
920	3.73	3.97
940	3.55	3.74

Hydrograph Volume = 8.780 acft

Hydraflow Hydrographs by Intelisolve

Hydrograph Discharge Table

Time	Inflow	Outflow
(min)	cfs	cfs
(min)	CIS	CTS
960	3.38	3.54
980	3.25	3.37
1000	3.18	3.26
1020	3.12	3.18
1040	3.05	3.11
1060	2.99	3.05
1080	2.93	2.98
1100	2.86	2.92
1120	2.80	2.85
1140	2.73	2.79
1160	2.67	2.73
1180	2.60	2.66
1200	2.54	2.60
1220	2.50	2.54
1240	2.49	2.51
1260	2.47	2.49
1280	2.46	2.47
1300	2.45	2.46
1320	2.43	2.45
1340	2.42	2.43
1360	2.41	2.42
1380	2.40	2.41
1400	2.38	2.40
1400	2.38	2.40
1420	2.37	2.38
1440	2.36	2.37
1460	0.00	2.08



Hyd. No. 3

Econ. Ditch Outlet

Hydrograph type	= Reservoir	Peak discharge	= 19.30 cfs
Storm frequency	= 1000 yrs	Time interval	= 4 min
Inflow hyd. No.	= 2	Reservoir name	= Economizer Outlet
Max. Elevation	= 547.51 ft	Max. Storage	= 0.221 acft

Storage Indication method used.

Hydrograph Discharge Table

	Time	Inflow	Elevation	Clv A	Clv B	Clv C	Clv D	Wr A	Wr B	Wr C	Wr D	Exfil	Outflow
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	(min)	cfs	ft	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	280	1.98	544.75										1.97
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	300	2.05	544.76										2.04
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	320	2.13	544.77										2.11
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	340	2.20	544.78										2.18
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	360	2.27	544.79										2.26
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	380	2.34	544.80										2.33
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	400	2.42	544.81										2.40
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	420	2.49	544.82										2.47
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	440	2.56	544.83										2.55
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	460	2.63	544.84										2.62
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	480	2.70	544.85										2.69
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	500	2.78	544.86										2.77
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	520	2.96	544.88										2.92
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	540	3.18	544.91										3.14
	560	3.41	544.94										3.37
	580	3.52	544.96										3.51
	600	3.70	544.98										3.66
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	620	4.04	545.02										3.94
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	640	4.54	545.11										4.39
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	660	5.24	545.24										5.01
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	680	6.08	545.43										5.73
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	700	8.13	545.83										7.10
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	720	15.66	547.01										14.24
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	740	18.28	547.37										17.87
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	760	19.01	547.47										18.89
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	780	19.22	547.50										19.19
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	800	19.28	547.51										19.27
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	820	19.29	547.51										19.29
880 5.17 545.62 6.41 900 4.33 545.17 6.41 920 3.97 545.05 4.63 940 3.74 544.99 4.07 960 3.54 544.97 3.79 980 3.37 544.94 3.40 1000 3.26 544.93 3.27	840	16.04	547.41										18.31
900 4.33 545.17 4.63 920 3.97 545.05 4.07 940 3.74 544.99 4.07 960 3.54 544.97 3.79 980 3.37 544.94 3.40 1000 3.26 544.93 3.27	860	7.69	546.49										8.86
920 3.97 545.05 4.07 940 3.74 544.99 3.79 960 3.54 544.97 3.57 980 3.37 544.94 3.40 1000 3.26 544.93 3.27	880	5.17	545.62										6.41
940 3.74 544.99 3.79 960 3.54 544.97 3.57 980 3.37 544.94 3.40 1000 3.26 544.93 3.27	900	4.33	545.17										4.63
960 3.54 544.97 3.57 980 3.37 544.94 3.40 1000 3.26 544.93 3.27	920	3.97	545.05										4.07
980 3.37 544.94 3.40 1000 3.26 544.93 3.27	940	3.74	544.99										3.79
980 3.37 544.94 3.40 1000 3.26 544.93 3.27	960		544.97										
	980		544.94										
1020 3.18 544.91 3.19	1000	3.26	544.93										3.27
	1020	3.18	544.91										3.19

Continues on next page...

Outflow hydrograph volume = 8.698 acft

Page 2

Hydrograph Discharge Table

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
()	010		010	010	010	013	010	010	010	010	010	010
1040	3.11	544.90										3.12
1060	3.05	544.90										3.06
1080	2.98	544.89										2.99
1100	2.92	544.88										2.93
1120	2.85	544.87										2.87
1140	2.79	544.86										2.80
1160	2.73	544.85										2.74
1180	2.66	544.84										2.67
1200	2.60	544.84										2.61
1220	2.54	544.83										2.55
1240	2.51	544.82										2.51
1260	2.49	544.82										2.49
1280	2.47	544.82										2.47
1300	2.46	544.81										2.46
1320	2.45	544.81										2.45
1340	2.43	544.81										2.43
1360	2.42	544.81										2.42
1380	2.41	544.81										2.41
1400	2.40	544.81										2.40
1420	2.38	544.80										2.38
1440	2.37	544.80										2.37
1460	2.08	544.78										2.19

Reservoir No. 1 - Economizer Outlet

Pond Data

Pond storage is based on known values

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (acft)	Total storage (acft)
0.00	541.00	00	0.000	0.000
0.75	541.75	00	0.009	0.009
1.50	542.50	00	0.012	0.021
2.25	543.25	00	0.016	0.037
3.00	544.00	00	0.020	0.057
3.50	544.50	00	0.016	0.073
3.75	544.75	00	0.009	0.082
4.00	545.00	00	0.009	0.091
4.25	545.25	00	0.010	0.101
4.50	545.50	00	0.011	0.112
4.75	545.75	00	0.011	0.123
5.00	546.00	00	0.012	0.135
5.25	546.25	00	0.012	0.147
5.50	546.50	00	0.013	0.160
5.75	546.75	00	0.014	0.174
6.00	547.00	00	0.015	0.189
6.25	547.25	00	0.015	0.204
6.50	547.50	00	0.016	0.220
6.75	547.75	00	0.017	0.237
7.00	548.00	00	0.018	0.255

								Note: A	Il outflows hav	ve been analyz	ed under inlet and	d outlet control.
Stage /	Storage / I	Discharge ⁻	Table									
Stage	Storage	Elevation	Clv A	Clv B	Clv C	Clv D	Wr A	Wr B	Wr C	Wr D	Exfil	Total
ft	acft	ft	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs
0.00	0.000	541.00	0.00	0.00			0.00					0.00
0.08	0.001	541.08	0.00	0.00			0.00					0.00
0.15	0.002	541.15	0.00	0.00			0.00					0.00
0.23	0.003	541.23	0.00	0.00			0.00					0.00
0.30	0.004	541.30	0.00	0.00			0.00					0.00
0.38	0.005	541.38	0.00	0.00			0.00					0.00
0.45	0.005	541.45	0.00	0.00			0.00					0.00
0.53	0.006	541.53	0.00	0.00			0.00					0.00
0.60	0.007	541.60	0.00	0.00			0.00					0.00
0.68	0.008	541.68	0.00	0.00			0.00					0.00
0.75	0.009	541.75	0.00	0.00			0.00					0.00
0.83	0.010	541.83	0.00	0.00			0.00					0.00
0.90	0.011	541.90	0.00	0.00			0.00					0.00
0.98	0.013	541.98	0.00	0.00			0.00					0.00
1.05	0.014	542.05	0.00	0.00			0.00					0.00
1.13	0.015	542.13	0.00	0.00			0.00					0.00
1.20	0.016	542.20	0.00	0.00			0.00					0.00
1.28	0.017	542.28	0.00	0.00			0.00					0.00

Continues on next page...

Hydraflow Hydrographs by Intelisolve

Economizer Outlet Stage / Storage / Discharge Table

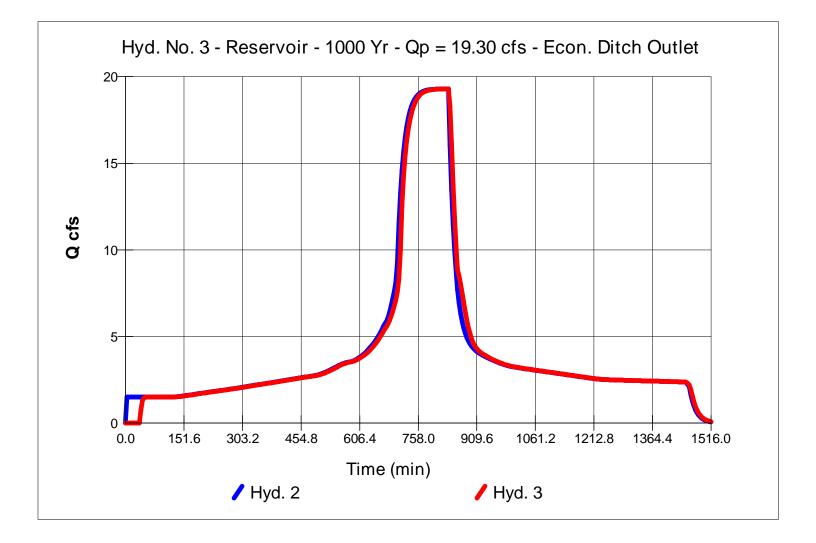
Stage /	Storage / I	Jischarge	lable									
Stage ft	Storage acft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Total cfs
1.35	0.019	542.35	0.00	0.00			0.00					0.00
1.43	0.020	542.43	0.00	0.00			0.00					0.00
1.50	0.021	542.50	0.00	0.00			0.00					0.00
1.58	0.023	542.58	0.00	0.00			0.00					0.00
1.65	0.024	542.65	0.00	0.00			0.00					0.00
1.73	0.026	542.73	0.00	0.00			0.00					0.00
1.80	0.027	542.80	0.00	0.00			0.00					0.00
1.88 1.95	0.029 0.031	542.88 542.95	0.00 0.00	0.00 0.00			0.00 0.00					0.00 0.00
2.03	0.031	543.03	0.00	0.00			0.00					0.00
2.10	0.034	543.10	0.00	0.00			0.00					0.00
2.18	0.035	543.18	0.00	0.00			0.00					0.00
2.25	0.037	543.25	0.00	0.00			0.00					0.00
2.33	0.039	543.33	0.00	0.00			0.00					0.00
2.40	0.041	543.40	0.00	0.00			0.00					0.00
2.48	0.043	543.48	0.00	0.00			0.00					0.00
2.55	0.045	543.55	0.00	0.00			0.00					0.00
2.63	0.047	543.63 543.70	0.00	0.00			0.00					0.00
2.70 2.78	0.049 0.051	543.70 543.78	0.00 0.00	0.00 0.00			0.00 0.00					0.00 0.00
2.78	0.053	543.85	0.00	0.00			0.00					0.00
2.93	0.055	543.93	0.00	0.00			0.00					0.00
3.00	0.057	544.00	0.00	0.00			0.00					0.00
3.05	0.059	544.05	0.00	0.00			0.00					0.00
3.10	0.060	544.10	0.00	0.00			0.00					0.00
3.15	0.062	544.15	0.00	0.00			0.00					0.00
3.20	0.063	544.20	0.00	0.00			0.00					0.00
3.25	0.065	544.25	0.00	0.00			0.00					0.00
3.30 3.35	0.067	544.30 544.35	0.00	0.00			0.00					0.00
3.35	0.068 0.070	544.35 544.40	0.00 0.00	0.00 0.00			0.00 0.00					0.00 0.00
3.40	0.070	544.45	0.00	0.00			0.00					0.00
3.50	0.073	544.50	0.00	0.00			0.00					0.00
3.53	0.074	544.53	0.00	0.00			0.00					0.20
3.55	0.075	544.55	0.00	0.00			0.00					0.40
3.58	0.076	544.58	0.00	0.00			0.00					0.59
3.60	0.077	544.60	0.00	0.00			0.00					0.79
3.63	0.077	544.63	0.00	0.00			0.00					0.99
3.65	0.078	544.65	0.00	0.00			0.00					1.19
3.68 3.70	0.079 0.080	544.68 544.70	0.00 0.00	0.00 0.00			0.00 0.00					1.39 1.58
3.70	0.080	544.70 544.73	0.00	0.00			0.00					1.56
3.75	0.082	544.75	0.00	0.00			0.00					1.98
3.78	0.083	544.78	0.00	0.00			0.00					2.17
3.80	0.084	544.80	0.00	0.00			0.00					2.35
3.83	0.085	544.83	0.00	0.00			0.00					2.54
3.85	0.086	544.85	0.00	0.00			0.00					2.72
3.88	0.086	544.88	0.00	0.00			0.00					2.91
3.90	0.087	544.90	0.00	0.00			0.00					3.09
3.93	0.088	544.93	0.00	0.00			0.00					3.28
3.95 3.98	0.089 0.090	544.95 544.98	0.00	0.00 0.00			0.00					3.46
4.00	0.090	544.98 545.00	0.00 0.00	0.00			0.00 0.00					3.65 3.83
4.00	0.091	545.03	0.00	0.00			0.00					3.95
4.05	0.093	545.05	0.00	0.00			0.00					4.07
4.08	0.094	545.08	0.00	0.00			0.00					4.19
4.10	0.095	545.10	0.00	0.00			0.00					4.31
4.13	0.096	545.13	0.00	0.00			0.00					4.43
4.15	0.097	545.15	0.00	0.00			0.00					4.56
4.18	0.098	545.18	0.00	0.00			0.00					4.68
4.20	0.099	545.20	0.00	0.00			0.00					4.80
4.23	0.100	545.23	0.00	0.00			0.00					4.92
4.25 4.28	0.101 0.102	545.25 545.28	0.00 0.00	0.00 0.00			0.00 0.00					5.04 5.14
4.28 4.30	0.102	545.28 545.30	0.00	0.00			0.00					5.14 5.23
4.30	0.103	545.33	0.00	0.00			0.00					5.33
4.35	0.104	545.35	0.00	0.00			0.00					5.43
4.38	0.107	545.38	0.00	0.00			0.00					5.53
4.40	0.108	545.40	0.00	0.00			0.00					5.62
4.43	0.109	545.43	0.00	0.00			0.00					5.72

Economizer Outlet Stage / Storage / Discharge Table

Stage /	Storage / I	Jischarge	lable									
Stage ft	Storage acft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Total cfs
4.45	0.110	545.45	0.00	0.00			0.00					5.82
4.48	0.111	545.48	0.00	0.00			0.00					5.91
4.50	0.112	545.50	0.00	0.00			0.00					6.01
4.53	0.113	545.53	0.00	0.00			0.00					6.09
4.55	0.114	545.55	0.00	0.00			0.00					6.18
4.58	0.115	545.58	0.00	0.00			0.00					6.26
4.60	0.116	545.60	0.00	0.00			0.00					6.35
4.63	0.118	545.63	0.00	0.00			0.00					6.43
4.65 4.68	0.119 0.120	545.65 545.68	0.00 0.00	0.00 0.00			0.00 0.00					6.51 6.60
4.70	0.120	545.70	0.00	0.00			0.00					6.68
4.73	0.122	545.73	0.00	0.00			0.00					6.77
4.75	0.123	545.75	0.00	0.00			0.00					6.85
4.78	0.124	545.78	0.00	0.00			0.00					6.92
4.80	0.125	545.80	0.00	0.00			0.00					7.00
4.83	0.127	545.83	0.00	0.00			0.00					7.07
4.85	0.128	545.85	0.00	0.00			0.00					7.15
4.88	0.129	545.88	0.00	0.00			0.00					7.22
4.90 4.93	0.130 0.131	545.90 545.93	0.00 0.00	0.00 0.00			0.00 0.00					7.29 7.37
4.93	0.131	545.95 545.95	0.00	0.00			0.00					7.44
4.98	0.134	545.98	0.00	0.00			0.00					7.52
5.00	0.135	546.00	0.00	0.00			0.00					7.59
5.03	0.136	546.03	0.00	0.00			0.00					7.66
5.05	0.137	546.05	0.00	0.00			0.00					7.73
5.08	0.139	546.08	0.00	0.00			0.00					7.79
5.10	0.140	546.10	0.00	0.00			0.00					7.86
5.13	0.141	546.13	0.00	0.00			0.00					7.93
5.15	0.142	546.15	0.00	0.00			0.00					8.00
5.18 5.20	0.143 0.145	546.18 546.20	0.00 0.00	0.00 0.00			0.00 0.00					8.07 8.13
5.20	0.145	546.23	0.00	0.00			0.00					8.20
5.25	0.140	546.25	0.00	0.00			0.00					8.27
5.28	0.148	546.28	0.00	0.00			0.00					8.33
5.30	0.150	546.30	0.00	0.00			0.00					8.39
5.33	0.151	546.33	0.00	0.00			0.00					8.46
5.35	0.152	546.35	0.00	0.00			0.00					8.52
5.38	0.154	546.38	0.00	0.00			0.00					8.58
5.40	0.155	546.40	0.00	0.00			0.00					8.64
5.43 5.45	0.156 0.157	546.43 546.45	0.00 0.00	0.00 0.00			0.00 0.00					8.70 8.77
5.45 5.48	0.157	546.45 546.48	0.00	0.00			0.00					8.83
5.50	0.160	546.50	0.00	0.00			0.00					8.89
5.53	0.161	546.53	0.00	0.00			0.00					9.15
5.55	0.163	546.55	0.00	0.00			0.00					9.42
5.58	0.164	546.58	0.00	0.00			0.00					9.68
5.60	0.166	546.60	0.00	0.00			0.00					9.94
5.63	0.167	546.63	0.00	0.00			0.00					10.21
5.65	0.168	546.65	0.00	0.00			0.00					10.47
5.68	0.170	546.68 546.70	0.00	0.00 0.00			0.00					10.73 10.99
5.70 5.73	0.171 0.173	546.73	0.00 0.00	0.00			0.00 0.00					11.26
5.75	0.173	546.75	0.00	0.00			0.00					11.52
5.78	0.176	546.78	0.00	0.00			0.00					11.78
5.80	0.177	546.80	0.00	0.00			0.00					12.04
5.83	0.179	546.83	0.00	0.00			0.00					12.30
5.85	0.180	546.85	0.00	0.00			0.00					12.56
5.88	0.182	546.88	0.00	0.00			0.00					12.82
5.90	0.183	546.90	0.00	0.00			0.00					13.07
5.93	0.185	546.93	0.00	0.00			0.00					13.33
5.95 5.98	0.186 0.188	546.95 546.98	0.00	0.00 0.00			0.00 0.00					13.59 13.85
5.98 6.00	0.188	546.98 547.00	0.00 0.00	0.00			0.00					13.85
6.03	0.189	547.00 547.03	0.00	0.00			0.00					14.11
6.05	0.192	547.05	0.00	0.00			0.00					14.62
6.08	0.194	547.08	0.00	0.00			0.00					14.88
6.10	0.195	547.10	0.00	0.00			0.00					15.13
6.13	0.197	547.13	0.00	0.00			0.00					15.39
6.15	0.198	547.15	0.00	0.00			0.00					15.65
6.18	0.200	547.18	0.00	0.00			0.00					15.90

Economizer Outlet Stage / Storage / Discharge Table

Stage / 3	Storage / I	Jischarge	ladie									
Stage	Storage	Elevation	Clv A	Clv B	Clv C	Clv D	Wr A	Wr B	Wr C	Wr D	Exfil	Total
ft	acft	ft	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs
6.20	0.201	547.20	0.00	0.00			0.00					16.16
6.23	0.203	547.23	0.00	0.00			0.00					16.41
6.25	0.204	547.25	0.00	0.00			0.00					16.67
6.28	0.206	547.28	0.00	0.00			0.00					16.92
6.30	0.207	547.30	0.00	0.00			0.00					17.18
6.33	0.209	547.33	0.00	0.00			0.00					17.43
6.35	0.210	547.35	0.00	0.00			0.00					17.68
6.38	0.212	547.38	0.00	0.00			0.00					17.94
6.40	0.214	547.40	0.00	0.00			0.00					18.19
6.43	0.215	547.43	0.00	0.00			0.00					18.44
6.45	0.217	547.45	0.00	0.00			0.00					18.69
6.48	0.218	547.48	0.00	0.00			0.00					18.95
6.50	0.220	547.50	0.00	0.00			0.00					19.20
6.53	0.222	547.53	0.00	0.00			0.00					19.45
6.55	0.223	547.55	0.00	0.00			0.00					19.70
6.58	0.225	547.58	0.00	0.00			0.00					19.96
6.60	0.227	547.60	0.00	0.00			0.00					20.21
6.63	0.229	547.63	0.00	0.00			0.00					20.46
6.65	0.230	547.65	0.00	0.00			0.00					20.71
6.68	0.232	547.68	0.00	0.00			0.00					20.96
6.70	0.234	547.70	0.00	0.00			0.00					21.22
6.73	0.235	547.73	0.00	0.00			0.00					21.47
6.75	0.237	547.75	0.00	0.00			0.00					21.72
6.78	0.239	547.78	0.00	0.00			0.00					21.97
6.80	0.241	547.80	0.00	0.00			0.00					22.22
6.83	0.242	547.83	0.00	0.00			0.00					22.47
6.85	0.244	547.85	0.00	0.00			0.00					22.72
6.88	0.246	547.88	0.00	0.00			0.00					22.97
6.90	0.248	547.90	0.00	0.00			0.00					23.21
6.93	0.250	547.93	0.00	0.00			0.00					23.46
6.95	0.251	547.95	0.00	0.00			0.00					23.71
6.98	0.253	547.98	0.00	0.00			0.00					23.96
7.00	0.255	548.00	0.00	0.00			0.00					24.21



Burlington Station As Built - Economizer Ditch Outlet

Hydrograph 3 Input

SUMMERGED OUTLET

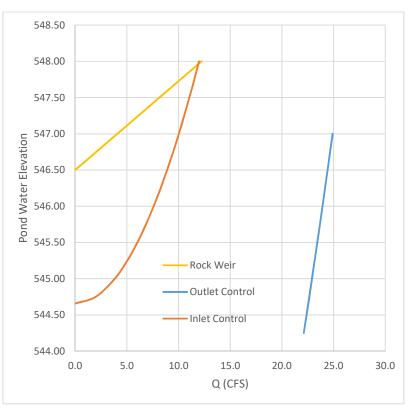
- 1.3167 FT = Pipe Diameter Steel
- 0.3292 FT = Hydraulic Radius
- $2.4998 \hspace{0.1in} k \hspace{0.1in} in \hspace{0.1in} Q \hspace{0.1in} outlet \hspace{0.1in} control \hspace{0.1in} equation$
- 544 FT = Pipe Inlet Invert Elevation
 - 65 FT = Length
- 0.011 = n for PVC
 - 0.6 = Co for Inlet Control
 - 534 FT = Tail Water Elevation
 - 12 FT = Weir Width
 - 0.5 Ft = d50
- 0.45 Porosity

OUTLET CONTROL / BARROW CONTROL

Q	Elevation	Head
CFS		Ft
22.12	544.25	10.3
22.38	544.50	10.5
22.65	544.75	10.8
22.91	545.00	11.0
23.17	545.25	11.3
23.43	545.50	11.5
23.68	545.75	11.8
23.93	546.00	12.0
24.18	546.25	12.3
24.42	546.50	12.5
24.67	546.75	12.8
24.91	547.00	13.0

INLET CONTROL

Q	Elevation	Head (to pipe middle)
CFS		Ft
0.00	544.66	0.00
1.98	544.75	0.09
3.83	545.00	0.34
5.04	545.25	0.59
6.01	545.50	0.84
6.85	545.75	1.09
7.59	546.00	1.34
8.27	546.25	1.59
8.89	546.50	1.84
9.48	546.75	2.09
10.03	547.00	2.34
10.55	547.25	2.59
11.05	547.50	2.84
11.52	547.75	3.09
11.98	548.00	3.34



Velocity Equation V = 2.48 * SQRT(G * d50) * (S^0.58 / Cu^2.22) Assume Cu = 2 1.51 FPS = V

Rock Filled V	Veir Flow		COMBINED	FLOW
Н	Q	Area	Н	Q
Ft	CFS	SF	Ft	CFS
546.50	0.0	0.0	544.66	0.00 Inlet Only
546.75	2.0	1.35	544.75	1.98
547.00	4.1	2.70	545.00	3.83
547.25	6.1	4.05	545.25	5.04
547.50	8.2	5.40	545.50	6.01
547.75	10.2	6.75	545.75	6.85
548.00	12.2	8.10	546.00	7.59
			546.25	8.27
Ignored side	slope rip-r	ap areas	546.50	8.89 Combined
			546.75	11.52
			547.00	14.11
			547.25	16.67
			547.50	19.20
			547.75	21.72
			548.00	24.21

Hydraflow Hydrographs by Intelisolve

Hyd. No. 4

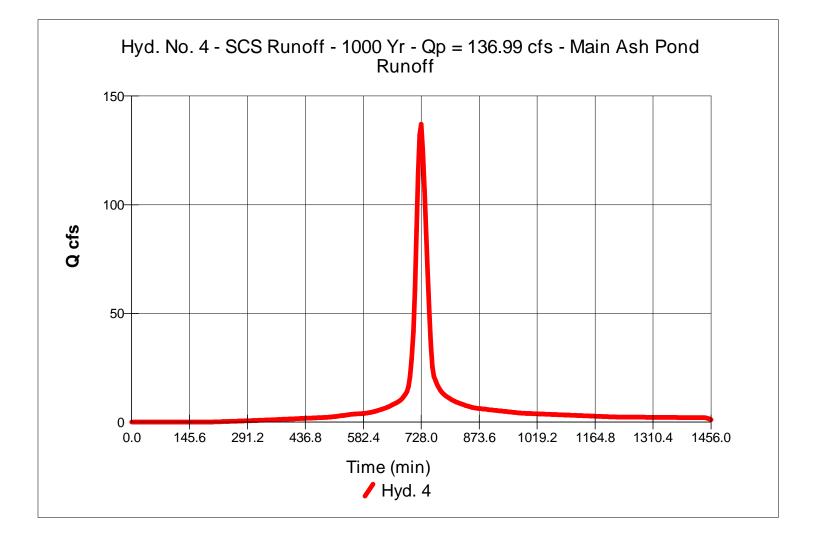
Main Ash Pond Runoff

Hydrograph type	= SCS Runoff	Peak discharge	= 136.99 cfs
Storm frequency	= 100 yrs	Time interval	= 4 min
Drainage area	= 17.00 ac	Curve number	= 85
Basin Slope	= 1.5 %	Hydraulic length	= 1250 ft
Tc method	= LAG	Time of conc. (Tc)	= 26.3 min
Total precip.	= 10.30 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484
		-	

Hydrograph Volume = 12.342 acft

Hydrograph Discharge Table

Time -- Outflow
(minCfs)69214.0271261.16732124.9975235.9777215.73



Hyd. No. 5

Main Base Flow

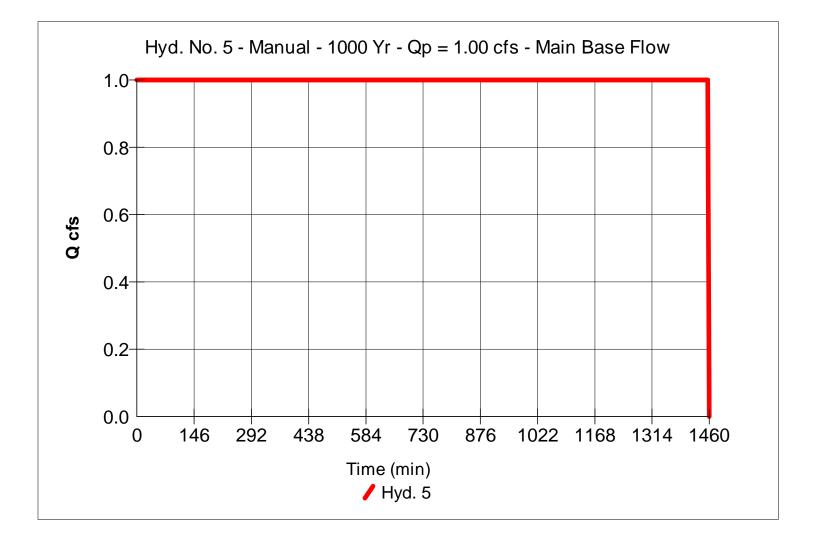
Hydrograph type	= Manual	Peak discharge	= 1.00 cfs
Storm frequency	= 1000 yrs	Time interval	= 4 min

Hydrograph Discharge Table

Time Outflow		Time	Outflow	Time Outflow		
(min	cfs)	(min	cfs)	(min	cfs)	
	,		·	·		
0	1.00 <<	680	1.00 <<	1360	1.00 <<	
20	1.00 <<	700	1.00 <<	1380	1.00 <<	
40	1.00 <<	720	1.00 <<	1400	1.00 <<	
60	1.00 <<	740	1.00 <<	1420	1.00 <<	
80	1.00 <<	760	1.00 <<	1440	1.00 <<	
100	1.00 <<	780	1.00 <<			
120	1.00 <<	800	1.00 <<			
140	1.00 <<	820	1.00 <<	End		
160	1.00 <<	840	1.00 <<			
180	1.00 <<	860	1.00 <<			
200	1.00 <<	880	1.00 <<			
220	1.00 <<	900	1.00 <<			
240	1.00 <<	920	1.00 <<			
260	1.00 <<	940	1.00 <<			
280	1.00 <<	960	1.00 <<			
300	1.00 <<	980	1.00 <<			
320	1.00 <<	1000	1.00 <<			
340	1.00 <<	1020	1.00 <<			
360	1.00 <<	1040	1.00 <<			
380	1.00 <<	1060	1.00 <<			
400	1.00 <<	1080	1.00 <<			
420	1.00 <<	1100	1.00 <<			
440	1.00 <<	1120	1.00 <<			
460	1.00 <<	1140	1.00 <<			
480	1.00 <<	1160	1.00 <<			
500	1.00 <<	1180	1.00 <<			
520	1.00 <<	1200	1.00 <<			
540	1.00 <<	1220	1.00 <<			
560	1.00 <<	1240	1.00 <<			
580	1.00 <<	1260	1.00 <<			
600	1.00 <<	1280	1.00 <<			
620	1.00 <<	1300	1.00 <<			
640	1.00 <<	1320	1.00 <<			
660	1.00 <<	1340	1.00 <<			

Hydrograph Volume = 2.011 acft

Hydraflow Hydrographs by Intelisolve



Hydrograph Report

Hydraflow Hydrographs by Intelisolve

Hyd. No. 6

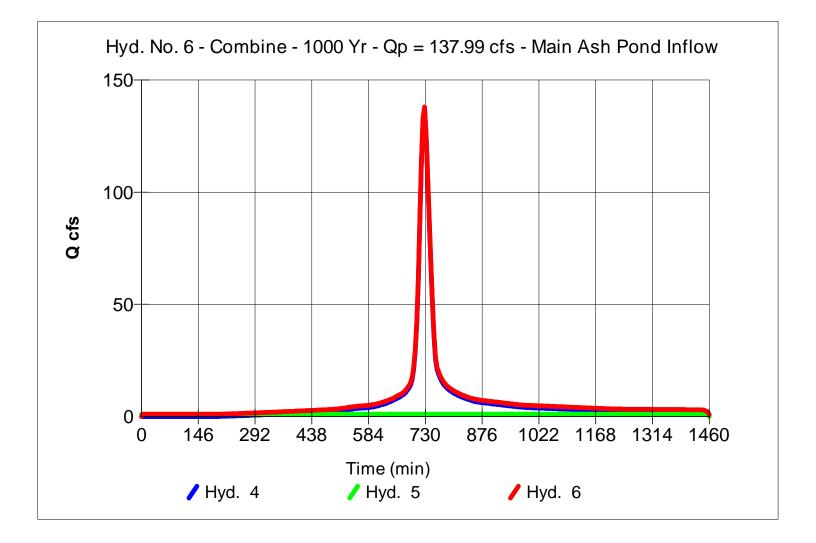
Main Ash Pond Inflow

Hydrograph type Storm frequency Inflow hyds.	= Combine = 1000 yrs = 4, 5	Peak discharge Time interval	= 137.99 cfs = 4 min
Hydrograph Disc	harge Table	Hydr	ograph Volume = 14.353 acft

Time	Hyd.4+	Hyd. 5 =	Outflow
(min)	(cfs)	(cfs)	(cfs)
700	20.99	1.00 <<	21.99
720	113.94	1.00 <<	114.94
740	86.98	1.00 <<	87.98
760	20.92	1.00 <<	21.92
780	13.66	1.00 <<	14.66

...End

Page 1



Outflow hydrograph volume = 14.348 acft

Hyd. No. 7

Main Ash Pond Outflow

Hydrograph type	= Reservoir	Peak discharge	= 13.71 cfs
Storm frequency	= 1000 yrs	Time interval	= 4 min
Inflow hyd. No.	= 6	Reservoir name	= Main Ash Pond
Max. Elevation	= 533.37 ft	Max. Storage	= 6.175 acft

Storage Indication method used.

Hydrograph Discharge Table

Time	Inflow	Elevation	Clv A	Clv B	Clv C	Clv D	Wr A	Wr B	Wr C	Wr D	Exfil	Outflow
(min)	cfs	ft	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs
300	1.65	531.42	1.39									1.39
320	1.79	531.44	1.52									1.52
340	1.94	531.46	1.65									1.65
360	2.09	531.47	1.80									1.80
380	2.25	531.49	1.96									1.96
400	2.41	531.51	2.12									2.12
420	2.58	531.53	2.28									2.28
440	2.74	531.55	2.44									2.44
460	2.91	531.56	2.61									2.61
480	3.07	531.58	2.78									2.78
500	3.31	531.60	2.96									2.96
520	3.74	531.63	3.22									3.22
540	4.23	531.66	3.58									3.58
560	4.67	531.71	4.00									4.00
580	4.90	531.74	4.36									4.36
600	5.43	531.78	4.73									4.73
620	6.35	531.83	5.27									5.27
640	7.59	531.91	6.05									6.05
660	9.37	532.00	6.89									6.89
680	11.90	532.04	7.15									7.15
700	21.99	532.11	7.72									7.72
720	114.94	532.57	9.77									9.77
740	87.98	533.23	13.10									13.10
760	21.92	533.35	13.64									13.64
780	14.66	533.37	13.71									13.71
800	11.75	533.37	13.70									13.70
820	9.94	533.36	13.65									13.65
840	8.58	533.34	13.58									13.58
860	7.59	533.32	13.50									13.50
880	7.07	533.30	13.40									13.40
900	6.67	533.27	13.30									13.30
920	6.26	533.25	13.19									13.19
940	5.85	533.22	13.08									13.08
960	5.44	533.20	12.96									12.96
980	5.09	533.17	12.83									12.83
1000	4.91	533.14	12.71									12.71
1020	4.76	533.11	12.58									12.58
1040	4.62	533.08	12.45									12.45

Hydrograph Discharge Table

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
1060	4.47	533.06	12.31									12.31
1080	4.32	533.03	12.18									12.18
1100	4.18	533.00	12.03									12.03
1120	4.03	532.92	11.68									11.68
1140	3.88	532.85	11.32									11.32
1160	3.74	532.79	10.97									10.97
1180	3.59	532.72	10.61									10.61
1200	3.44	532.66	10.26									10.26
1220	3.32	532.59	9.90									9.90
1240	3.28	532.53	9.54									9.54
1260	3.25	532.48	9.19									9.19
1280	3.22	532.42	8.85									8.85
1300	3.19	532.37	8.67									8.67
1320	3.16	532.32	8.62									8.62
1340	3.13	532.27	8.52									8.52
1360	3.10	532.22	8.37									8.37
1380	3.07	532.17	8.14									8.14
1400	3.04	532.13	7.85									7.85
1420	3.01	532.09	7.55									7.55
1440	2.98	532.05	7.22									7.22
1460	0.77	532.00	6.87									6.87
1480	0.00	531.70	3.94									3.94
1500	0.00	531.52	2.19									2.19

Reservoir No. 3 - Main Ash Pond

Pond Data

Pond storage is based on known contour areas. Average end area method used.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (acft)	Total storage (acft)
0.00	531.10	15,000	0.000	0.000
0.90	532.00	24,000	0.403	0.403
1.90	533.00	235,000	2.973	3.376
2.90	534.00	427,000	7.599	10.975

Weir Structures

Culvert / Orifice Structures

	[A]	[B]	[C]	[D]	
Rise in	= 15.0	0.0	0.0	0.0	Crest Len ft
Span in	= 15.0	0.0	0.0	0.0	Crest El. ft
No. Barrels	= 2	0	0	0	Weir Coeff.
Invert El. ft	= 531.10	0.00	0.00	0.00	Weir Type
Length ft	= 60.0	0.0	0.0	0.0	Multi-Stage
Slope %	= 1.00	0.00	0.00	0.00	
N-Value	= .015	.000	.000	.000	
Orif. Coeff.	= 0.86	0.00	0.00	0.00	
Multi-Stage	= n/a	No	No	No	Exfiltration R

[B]

0.00

0.00

0.00

No

[C]

0.00

0.00

0.00

No

[A]

= 0.00

= 0.00

= 0.00

= ----

= No

ion Rate = 0.00 in/hr/sqft Tailwater Elev. = 0.00 ft

Note: All outflows have been analyzed under inlet and outlet control.

Stage /	Storage / I	Discharge 7	Fable					Note. P	ui outilows nav	e been analyz	ed under miet and	
Stage	Storage	Elevation	Clv A	Clv B	Clv C	Clv D	Wr A	Wr B	Wr C	Wr D	Exfil	Total
ft	acft	ft	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs
0.00	0.000	531.10	0.00									0.00
0.00	0.000	531.10	0.00									0.00
		531.19	0.11									-
0.18	0.081											0.45
0.27	0.121	531.37	0.99									0.99
0.36	0.161	531.46	1.69									1.69
0.45	0.201	531.55	2.47									2.47
0.54	0.242	531.64	3.34									3.34
0.63	0.282	531.73	4.23									4.23
0.72	0.322	531.82	5.14									5.14
0.81	0.363	531.91	6.02									6.02
0.90	0.403	532.00	6.85									6.85
1.00	0.700	532.10	7.66									7.66
1.10	0.997	532.20	8.30									8.30
1.20	1.295	532.30	8.60									8.60
1.30	1.592	532.40	8.69									8.69
1.40	1.889	532.50	9.33									9.33
1.50	2.187	532.60	9.94									9.94
1.60	2.484	532.70	10.50									10.50
1.70	2.781	532.80	11.04									11.04
1.80	3.079	532.90	11.56									11.56
1.90	3.376	533.00	12.05									12.05
2.00	4.136	533.10	12.52									12.52
2.10	4.896	533.20	12.98									12.98
2.20	5.655	533.30	13.42									13.42
2.30	6.415	533.40	13.84									13.84
2.40	7.175	533.50	14.26									14.26
2.50	7.935	533.60	14.66									14.66
2.60	8.695	533.70	15.05									15.05
2.70	9.455	533.80	15.43									15.43
2.80	10.215	533.90	15.80									15.80
2.90	10.975	534.00	16.17									16.17
2.30	10.375	334.00	10.17									10.17

Hydraflow Hydrographs by Intelisolve

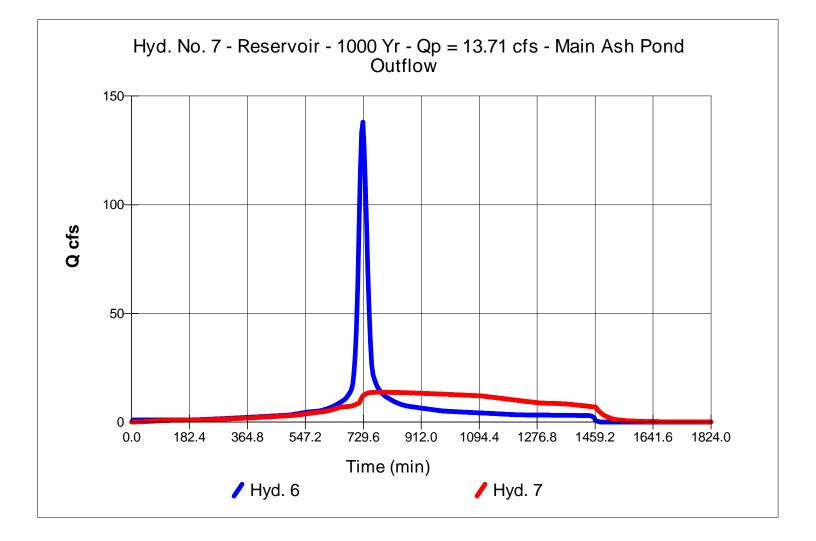
[D]

0.00

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No



Hydraflow Hydrographs by Intelisolve

Hyd. No. 8

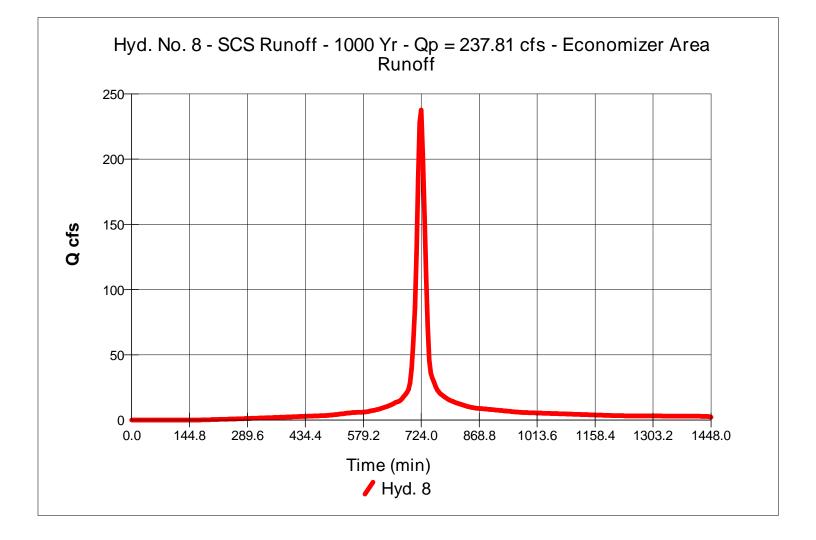
Economizer Area Runoff

Hydrograph type	= SCS Runoff	Peak discharge	= 237.81 cfs
Storm frequency	= 1000 yrs	Time interval	= 4 min
Drainage area	= 25.00 ac	Curve number	= 87
Basin Slope	= 1.0 %	Hydraulic length	= 700 ft
Tc method	= LAG	Time of conc. (Tc)	= 18.8 min
Total precip.	= 10.30 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

Hydrograph Volume = 18.127 acft

Hydrograph Discharge Table

Time -- Outflow
(minCfs)69628.55716183.34736110.2075628.60



Hydrograph Report

Hyd. No. 9

Upper Ash Pond Inflow

Hydrograph type Storm frequency Inflow hyds.	= Combine = 1000 yrs = 3, 7, 8	Peak discharge Time interval	= 264.17 cfs = 4 min
		Hydr	ograph Volume = 41.173 a

Hydrograph Discharge Table

Time (min)	Hyd.3+ (cfs)	Hyd. 7 + (cfs)	Hyd. 8 = (cfs)
(1111)	(013)	(013)	(013)
680	5.73	7.15	17.85
700	7.10	7.72	39.52
720	14.24	9.77	228.18
740	17.87	13.10	71.83
760	18.89	13.64	25.55
780	19.19	13.71	18.17
800	19.27	13.70	14.61
820	19.29	13.65	12.26
840	18.31	13.58	10.44
860	8.86	13.50	9.19
880	6.41	13.40	8.58

...End

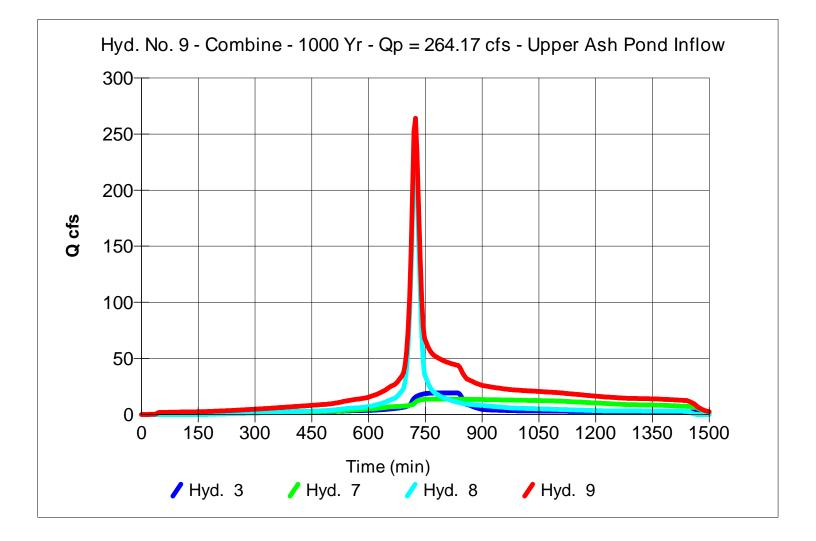


acft grap ıy

Outflow (cfs)

30.72 54.34 252.19 102.79 58.09 51.06 47.57 45.20 42.34 31.54 28.39

Page 1



Outflow hydrograph volume = 41.023 acft

Hyd. No. 10

Upper Ash Pond Outflow

Hydrograph type	= Reservoir	Peak discharge	= 9.35 cfs
Storm frequency	= 1000 yrs	Time interval	= 4 min
Inflow hyd. No.	= 9	Reservoir name	= Upper Ash Pond
Max. Elevation	= 530.23 ft	Max. Storage	= 29.621 acft

Storage Indication method used.

Hydrograph Discharge Table

Time	Inflow	Elevation	Clv A	Clv B	Clv C	Clv D	Wr A	Wr B	Wr C	Wr D	Exfil	Outflow
(min)	cfs	ft	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs
	4.00	500.04										4.00
280	4.32	528.01										1.02
300	4.72	528.03										1.14
320	5.14	528.04										1.28
340	5.57	528.06										1.43
360	6.03	528.08										1.59
380	6.49	528.10										1.76
400	6.96	528.11										1.89
420	7.43	528.12										2.01
440	7.91	528.13										2.14
460	8.39	528.14										2.27
480	8.88	528.16										2.41
500	9.55	528.17										2.57
520	10.66	528.19										2.74
540	11.99	528.20										2.93
560	13.20	528.22										3.15
580	13.99	528.24										3.38
600	15.50	528.27										3.64
620	17.80	528.29										3.93
640	20.99	528.33										4.30
660	25.35	528.36										4.77
680	30.72	528.41										5.32
700	54.34	528.48										6.13
720	252.19	528.75										7.66
740	102.79	529.14										8.09
760	58.09	529.27										8.23
780	51.06	529.37										8.34
800	47.57	529.45										8.43
820	45.20	529.53										8.51
840	42.34	529.60										8.59
860	31.54	529.66										8.66
880	28.39	529.70										8.71
900	25.92	529.74										8.75
920	24.66	529.78										8.79
940	23.68	529.81										8.83
960	22.76	529.84										8.87
980	22.01	529.87										8.90
1000	21.54	529.89										8.94
1000	21.12	529.92										8.97
1020	ZI.IZ	523.32										0.97

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
()	0.0		0.0	0.0	010	0.0	0.0	010	0.0	0.0	010	0.0
1040	20.71	529.94										9.00
1060	20.30	529.97										9.03
1080	19.89	529.99										9.06
1100	19.47	530.01										9.08
1120	18.84	530.03										9.11
1140	18.21	530.05										9.13
1160	17.58	530.07										9.16
1180	16.95	530.09										9.18
1200	16.31	530.10										9.20
1220	15.75	530.12										9.21
1240	15.31	530.13										9.23
1260	14.90	530.14										9.24
1280	14.49	530.15										9.26
1300	14.26	530.16										9.27
1320	14.16	530.17										9.28
1340	14.00	530.18										9.30
1360	13.79	530.19										9.31
1380	13.51	530.20										9.32
1400	13.16	530.21										9.33
1420	12.81	530.22										9.34
1440	12.42	530.22										9.35
1460	9.48	530.23 <<										9.35 <<
1480	4.68	530.22										9.34
1500	2.40	530.21										9.33
1520	1.38	530.20										9.31
1540	0.87	530.18										9.29
1560	0.59	530.16										9.27
1580	0.42	530.14										9.24
1600	0.34	530.12										9.22
1620	0.27	530.11										9.20
1640	0.21	530.09										9.18
1660	0.17	530.07										9.15
1680	0.13	530.05										9.13
1700	0.11	530.03										9.11
1720	0.10	530.01										9.08
1740	0.09	529.99										9.06
1760	0.09	529.98										9.04
1780	0.08 0.08	529.96										9.02 8.99
1800 1820		529.94 529.92										
1840	0.07 0.06	529.92 529.90										8.97 8.95
1860	0.06	529.88										8.93
1880	0.06	529.86										8.90
1900 1920	0.05	529.85										8.88
	0.05	529.83										8.86
1940	0.04	529.81										8.84
1960	0.04	529.79 520.77										8.81
1980	0.04	529.77 520.76										8.79 8.77
2000	0.03	529.76										8.77 9.75
2020	0.03	529.74 520.72										8.75
2040	0.03	529.72										8.72

$\begin{array}{cccccccccccccccccccccccccccccccccccc$	Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
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2740 0.00 529.11 8.06 2760 0.00 529.10 8.04 2780 0.00 529.08 8.02 2800 0.00 529.06 8.02 2820 0.00 529.05 8.00 2820 0.00 529.05 8.00 2840 0.00 529.03 7.99 2860 0.00 529.01 7.95 2880 0.00 529.00 7.93 2900 0.00 528.98 7.91 2920 0.00 528.95 7.84 2960													
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2840 0.00 529.03 7.97 2860 0.00 529.01 7.95 2880 0.00 529.00 7.93 2900 0.00 528.98 7.91 2920 0.00 528.96 7.91 2920 0.00 528.96 7.90 2940 0.00 528.95 7.88 2960 0.00 528.93 7.86 2980 0.00 528.91 7.84 3000 0.00 528.88 7.83 3020 0.00 528.87 7.81													
2860 0.00 529.01 7.95 2880 0.00 529.00 7.93 2900 0.00 528.98 7.91 2920 0.00 528.96 7.90 2940 0.00 528.95 7.90 2940 0.00 528.93 7.88 2960 0.00 528.93 7.86 2980 0.00 528.91 7.84 3000 0.00 528.88 7.83 3020 0.00 528.87 7.81 3040 0.00 528.87 7.79													
2880 0.00 529.00 7.93 2900 0.00 528.98 7.91 2920 0.00 528.96 7.90 2940 0.00 528.95 7.88 2960 0.00 528.93 7.86 2980 0.00 528.91 7.84 3000 0.00 528.88 7.83 3020 0.00 528.87 7.81 3040 0.00 528.87 7.79													
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2920 0.00 528.96 7.90 2940 0.00 528.95 7.88 2960 0.00 528.93 7.86 2980 0.00 528.91 7.84 3000 0.00 528.80 7.83 3020 0.00 528.88 7.81 3040 0.00 528.87 7.79													
2940 0.00 528.95 7.88 2960 0.00 528.93 7.86 2980 0.00 528.91 7.84 3000 0.00 528.90 7.83 3020 0.00 528.88 7.81 3040 0.00 528.87 7.79	2920		528.96										
2960 0.00 528.93 7.86 2980 0.00 528.91 7.84 3000 0.00 528.90 7.83 3020 0.00 528.88 7.81 3040 0.00 528.87 7.79													
2980 0.00 528.91 7.84 3000 0.00 528.90 7.83 3020 0.00 528.88 7.81 3040 0.00 528.87 7.79													
3000 0.00 528.90 7.83 3020 0.00 528.88 7.81 3040 0.00 528.87 7.79													
3020 0.00 528.88 7.81 3040 0.00 528.87 7.79													
3040 0.00 528.87													

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
3080	0.00	528.83										7.75
3100	0.00	528.82										7.74
3120	0.00	528.80										7.72
3140	0.00	528.79										7.70
3160	0.00	528.77										7.68
3180	0.00	528.75										7.67
3200	0.00	528.74										7.65
3220	0.00	528.72										7.63
3240	0.00	528.71										7.62
3260	0.00	528.69										7.60
3280	0.00	528.67										7.58
3300	0.00	528.66										7.56
3320	0.00	528.64										7.55
3340	0.00	528.63										7.53
3360	0.00	528.61										7.51
3380	0.00	528.60										7.46
3400	0.00	528.58										7.29
3420	0.00	528.57										7.13
3440	0.00	528.55										6.97
3460	0.00	528.54										6.81
3480	0.00	528.52										6.66
3500	0.00	528.51										6.51
3520	0.00	528.50										6.36
3540	0.00	528.48										6.20
3560	0.00	528.47										6.05
3580	0.00	528.46										5.90
3600	0.00	528.45										5.75
3620	0.00	528.43										5.61
3640	0.00	528.42										5.47
3660	0.00	528.41										5.34
3680	0.00	528.40										5.21
3700	0.00	528.39										5.08
3720	0.00	528.38										4.96
3740	0.00	528.37										4.83
3760	0.00	528.36										4.72
3780	0.00	528.35										4.60
3800	0.00	528.34										4.49
3820	0.00	528.33										4.38
3840	0.00	528.32										4.27
3860	0.00	528.31										4.16
3880	0.00	528.31										4.06
3900	0.00	528.30										3.97
3920	0.00	528.29										3.88
3940	0.00	528.28										3.79
3960	0.00	528.27										3.70
3980	0.00	528.27										3.62
4000	0.00	528.26										3.54
4020	0.00	528.25										3.46
4040	0.00	528.24										3.38
4060	0.00	528.24										3.30
4080	0.00	528.23										3.23

Time (min)	Inflow cfs	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	Clv D cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	Outflow cfs
44.00	0.00	E20.22										2.46
4100 4120	0.00 0.00	528.22 528.22										3.16 3.09
4120	0.00	528.21										3.09
4140	0.00	528.20										3.02 2.95
4180	0.00	528.20										2.95
4180	0.00	528.19										2.80
4200	0.00	528.19										2.82
4240	0.00	528.18										2.69
4260	0.00	528.18										2.63
4280	0.00	528.17										2.57
4300	0.00	528.16										2.51
4320	0.00	528.16										2.46
4340	0.00	528.15										2.40
4360	0.00	528.15										2.35
4380	0.00	528.15										2.29
4400	0.00	528.14										2.24
4420	0.00	528.14										2.19
4440	0.00	528.13										2.14
4460	0.00	528.13										2.09
4480	0.00	528.12										2.05
4500	0.00	528.12										2.00
4520	0.00	528.11										1.96
4540	0.00	528.11										1.91
4560	0.00	528.11										1.87
4580	0.00	528.10										1.83
4600	0.00	528.10										1.78
4620	0.00	528.09										1.71
4640	0.00	528.08										1.65
4660	0.00	528.08										1.59
4680	0.00	528.07										1.53
4700	0.00	528.06										1.47
4720	0.00	528.06										1.42
4740	0.00	528.05										1.37
4760	0.00	528.05										1.32
4780	0.00	528.04										1.27
4800	0.00	528.04										1.22
4820	0.00	528.03										1.18
4840	0.00	528.03										1.14
4860	0.00	528.02										1.09
4880	0.00	528.02										1.05
4900	0.00	528.01										1.02
4920	0.00	528.01										0.98
4940	0.00	528.00										0.94
	0.00	0_0100										0.01

Reservoir No. 2 - Upper Ash Pond

Pond Data

Pond storage is based on known contour areas. Average end area method used.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (acft)	Total storage (acft)
0.00	527.90	00	0.000	0.000
0.20	528.10	579,000	1.329	1.329
0.40	528.30	579,000	2.658	3.988
0.60	528.50	579,000	2.658	6.646
0.70	528.60	579,000	1.329	7.975
1.80	529.70	579,000	14.621	22.596
3.10	531.00	579,000	17.280	39.876
4.00	531.90	579,000	11.963	51.839

Weir Structures

Culvert / Orifice Structures

	[A]	[B]	[C]	[D]		[A]	[B]	[C]	[D]
Rise in	= 0.0	0.0	0.0	0.0	Crest Len ft	= 0.00	0.00	0.00	0.00
Span in	= 0.0	0.0	0.0	0.0	Crest El. ft	= 0.00	0.00	0.00	0.00
No. Barrels	= 0	0	0	0	Weir Coeff.	= 0.00	0.00	0.00	0.00
Invert El. ft	= 0.00	0.00	0.00	0.00	Weir Type	=			
Length ft	= 0.0	0.0	0.0	0.0	Multi-Stage	= No	No	No	No
Slope %	= 0.00	0.00	0.00	0.00					
N-Value	= .013	.000	.000	.000					
Orif. Coeff.	= 0.60	0.00	0.00	0.00					
Multi-Stage	= n/a	No	No	No	Exfiltration Rat	e = 0.00 in/hr/	sqft Tailwa	ater Elev. =	• 0.00 ft

Stage / Storage / Discharge Table

Stage Storage Elevation CIv A Clv B Clv C Clv D Wr A Wr B Wr C Wr D Exfil Total ft acft ft cfs 0.00 0.000 527.90 -------------------------------0.00 0.02 0.133 527.92 ---0.18 --------------------------0.04 527.94 -------------------------0.36 0.266 -------0.06 0.399 527.96 ------------------------------0.54 0.08 0.532 527.98 0.72 ------------------------------0.10 0.665 528.00 ----------------------------------0.90 0.798 528.02 ------------1.08 0.12 ------------------0.14 0.930 528.04 --------------------------1.26 0.16 1.063 528.06 ----------------------------------1.44 1.196 528.08 1.62 0.18 -----------------------------------0.20 1.329 528.10 ----------------------1.80 0.22 1.595 528.12 ----------------2.02 ------------0.24 1.861 528.14 ----2.24 ---------------------------0.26 2.127 528.16 --------------------------------2.46 528.18 2.68 0.28 2.393 --------------------------------0.30 2.658 528.20 ---------------------------2.90 -------------------3.12 0.32 2.924 528.22 -----------3.190 --------3.34 0.34 528.24 -----------------------0.36 3.456 528.26 ----------------------------3.56 ---0.38 3.722 528.28 ------------------------------3.78 0.40 3.988 528.30 ------------------------------4.00 0.42 4.253 528.32 ---------------------------4.24 ------------4.48 0.44 4.519 528.34 --------------------------0.46 4.785 528.36 ------------------------------4.72 5.051 --------------------------------4.96 0.48 528.38 0.50 5.317 528.40 -------------------------------5.20 5.583 528.42 5.44 0.52 -------------------------------0.54 5.848 528.44 -----------------------------------5.68 0.56 6.114 528.46 ------------------------------5.92 0.58 6.380 528.48 -------------------6.16 -----------

Continues on next page ...

Hydraflow Hydrographs by Intelisolve

Note: All outflows have been analyzed under inlet and outlet control.

Upper Asl		<u>.</u>	.									Page 2
Stage /	Storage / I	Discharge ⁻	Table									
Stage	Storage	Elevation	CIV A	Clv B	Clv C	Clv D	Wr A	Wr B	Wr C	Wr D	Exfil	Total
ft	acft	ft	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs
0.60	6.646	528.50										6.40
0.60	6.779	528.50										6.51
0.62	6.912	528.52										6.62
0.63	7.045	528.53										6.73
0.64	7.178	528.54										6.84
0.65	7.311	528.55										6.95
0.66	7.444	528.56										7.06
0.67	7.576	528.57										7.17
0.68	7.709	528.58										7.28
0.69	7.842	528.59										7.39
0.70	7.975	528.60										7.50
0.81	9.437	528.71										7.62
0.92	10.899	528.82										7.74
1.03	12.362	528.93										7.86
1.14	13.824	529.04										7.98
1.25	15.286	529.15										8.10
1.36	16.748	529.26										8.22
1.47	18.210	529.37										8.34
1.58	19.672	529.48										8.46
1.69	21.134	529.59										8.58
1.80	22.596	529.70										8.70
1.93	24.324	529.83										8.86
2.06	26.052	529.96										9.02
2.19	27.780	530.09										9.18
2.32	29.508	530.22										9.34
2.45	31.236	530.35										9.50
2.58	32.964	530.48										9.66
2.71	34.692	530.61										9.82
2.84	36.420	530.74										9.98
2.97 3.10	38.148	530.87 531.00										10.14 10.30
	39.876											
3.19 3.28	41.072 42.269	531.09 531.18										10.37 10.44
3.28 3.37	42.269 43.465	531.16										10.44
3.37	43.405 44.661	531.36										10.51
3.40	44.001	531.36										10.58
3.64	45.657 47.054	531.54										10.05
3.73	48.250	531.63										10.72
3.73	48.250 49.446	531.63										10.79
3.82 3.91	49.440 50.643	531.81										10.88
4.00	51.839	531.90										11.00
4.00	51.059	551.80										11.00

...End

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Upper Ash Pond Stage / Storag o / Disch

